

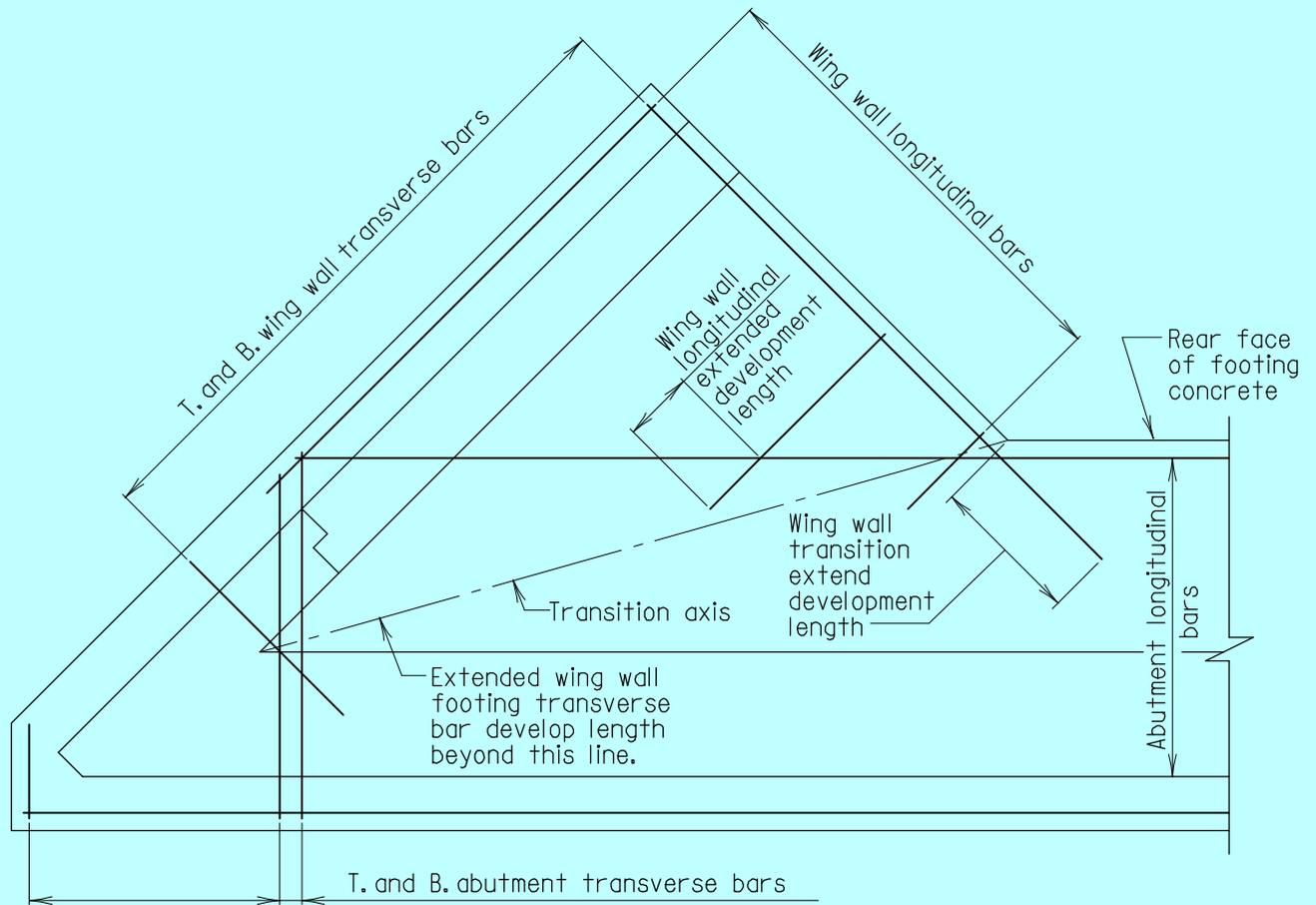
FOUNDATIONS

(FND)

FOUNDATIONS

GENERAL

(FND-GN)



ACUTE CORNER

Scale: $\frac{3}{8}'' = 1'-0''$

* FOR OFFICE USE ONLY *

APPROVAL	
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 11/17/97	
REVISIONS	
SHA	FHWA

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

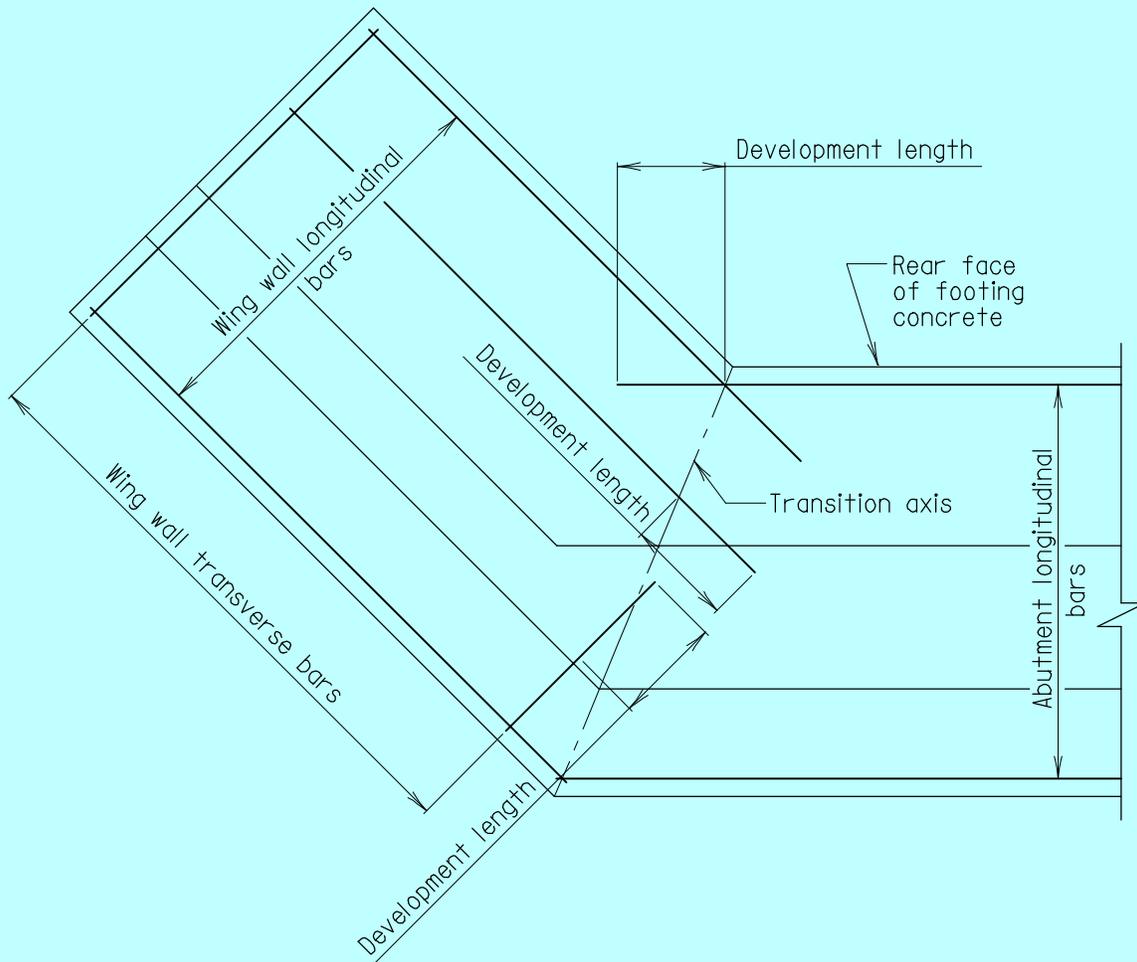
SKEWED ABUTMENT AND WING WALL
FOOTING INTERSECTION

STANDARD NO. FND-GN-101

SHEET 1 OF 2

FHWA APPROVAL
DATE:

FOUNDATION - GENERAL



OBTUSE CORNER

Scale: $\frac{3}{8}'' = 1'-0''$

* FOR OFFICE USE ONLY *

APPROVAL	
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES DATE: 11/17/97	
REVISIONS	
SHA	FHWA

STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

SKEWED ABUTMENT AND WING WALL
 FOOTING INTERSECTION

STANDARD NO. FND-GN-101	SHEET <u>2</u> OF <u>2</u>
-------------------------	----------------------------

FHWA APPROVAL	
DATE:	

FOUNDATION - GENERAL

FOUNDATIONS

PILE FOUNDATIONS

(FND-PF)

Limits of temporary fill during compaction of preliminary embankment. Temporary fill within these limits to be removed to finished slope line and used in completing preliminary embankment after abutment is completed. Removal of this overburden and placing of same behind abutments will be measured and paid for as Class 2 Excavation.

Limits of completed preliminary embankment

150' (Min.) for approaches to bridges; unless cut is encountered sooner (measured parallel to ϕ of roadway).

Bottom of subgrade for roadway pavement.

Proposed roadway surface

Proposed abutment

8'-0"

* Proposed finished slope 2:1 unless otherwise designated.

** Slope ratio depends upon fill height.

Slope to drain

Limit of preliminary embankment prior to driving piles for abutments.

25'

See Note 'A'

Finished Groundline

Existing Groundline

Finished slope or top of proposed slope protection.

Slope top of preliminary embankment to drain to ϕ of embankment (i.e. midway between outside shoulders) and from abutment to rear of fill along ϕ , to carry drainage down rear embankment slope to sediment trap or other erosion control device.

Temporarily seed and mulch front and back slopes to original groundline. Permanent seed and mulch on side slopes. Install 4'-0" wide soil stabilization matting in top swale to original groundline.

ELEVATION

Scale: None

Note A:
No boulders, rocks, or stumps in this area of fill and all stumps, surface boulders and rocks to be removed from existing ground within these limits.

APPROVAL	
<i>L.S. Fisher</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 11/8/76	
REVISIONS	
SHA	FHWA
11-9-76	11-9-76
4-28-94	
FHWA APPROVAL	3-20-01
DATE: 11-9-76	7-24-01

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

PRELIMINARY EMBANKMENT FOR PEDESTAL
TYPE BRIDGE ABUTMENTS ON PILES

STANDARD NO. FND-PF-101

SHEET 1 OF 1

OLD NO. BR-FD(0.10)-75-17

FOUNDATION - PILES

Limits of temporary fill during compaction of preliminary embankment. Temporary fill within these limits to be removed to finished slope line and used in completing preliminary embankment after abutment is completed. Removal of this overburden and placing of same behind abutments will be measured and paid for as Class 2 Excavation.

Area to be excavated prior to driving piles (shown double hatched to be paid for as Class 3 Excavation.

150' (Min.) for approaches to bridges; unless cut is encountered sooner (measured parallel to ϕ of roadway).

Bottom of subgrade inside of wing wall.

Bottom of subgrade for roadway pavement.

Abutment stem

Front face of abutment footing.

Rear of abutment footing.

Limit of preliminary embankment prior to driving piles for abutment and wings.

1'-6"

8'-0"

1'-6"

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1'-6"

Finished groundline outside of wing wall.

End of wing wall.

Limit of preliminary embankment.

Proposed roadway surface.

Slope to drain

Limit of preliminary embankments prior to driving piles for abutments and wings.

Slope as steep as ground will allow.

25'

Existing groundline

Slope top of preliminary embankment to drain to ϕ of embankment (i.e. midway between outside shoulders) and from abutment to rear of fill along ϕ , to carry drainage down rear embankment slope to sediment trap or other erosion control device.

Temporarily seed and mulch front and back slopes to original groundline. Permanent seed and mulch on side slopes. Install 4'-0" wide soil stabilization matting in top swale to original groundline.

ELEVATION

Scale: None

See Note 'A'

Finished slope or top of proposed slope protection.

Finished groundline

* Proposed finished slope 2:1 unless otherwise designated.

* Slope ratio depends upon fill height.

Note A:

No boulders, rocks, or stumps in this area of fill and all stumps, surface boulders and rocks to be removed from existing ground within these limits.

APPROVAL	
<i>L.S. Fisher</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 9/6/85	
REVISIONS	
SHA	FHWA
6-23-87	6-8-90
4-27-94	

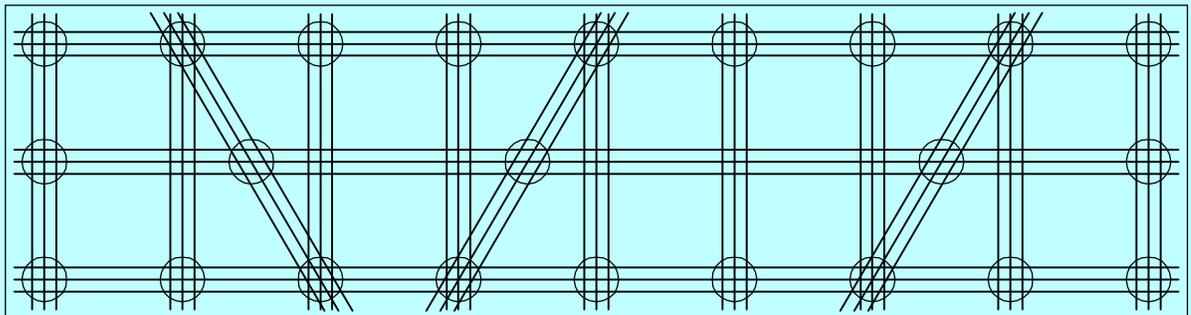
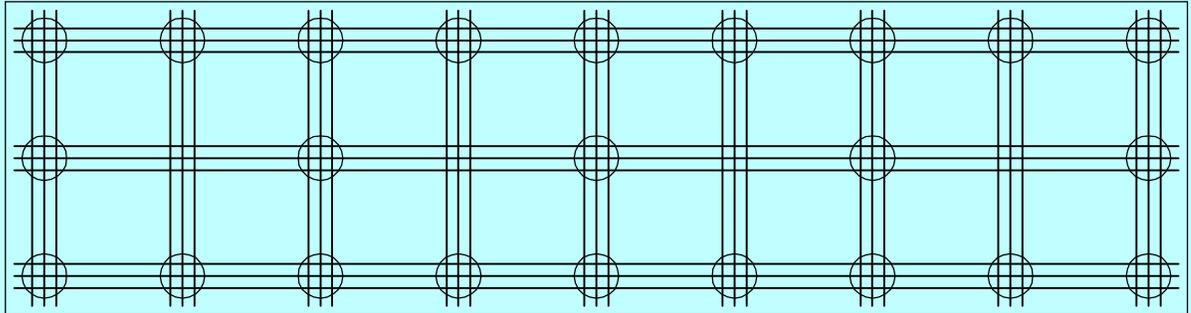
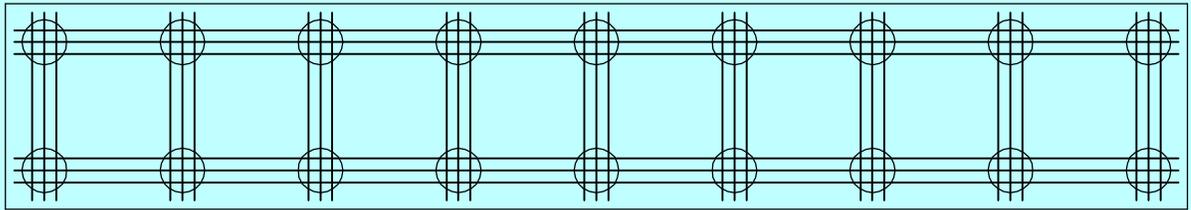
FHWA APPROVAL	3-20-01
DATE: 6-8-90	7-24-01

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

PRELIMINARY EMBANKMENT FOR SEMI-CANTILEVER TYPE ABUTMENTS ON PILES WITH STEPPED WING WALL FOOTERS

STANDARD NO. FND-PF-102

SHEET OF



TYPICAL PIER FOOTING PLANS

Scale: $\frac{3}{16}''=1'-0''$

Notes:

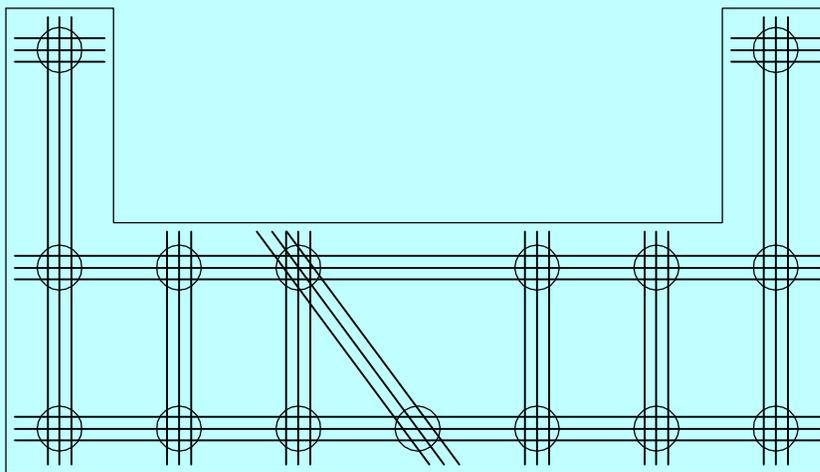
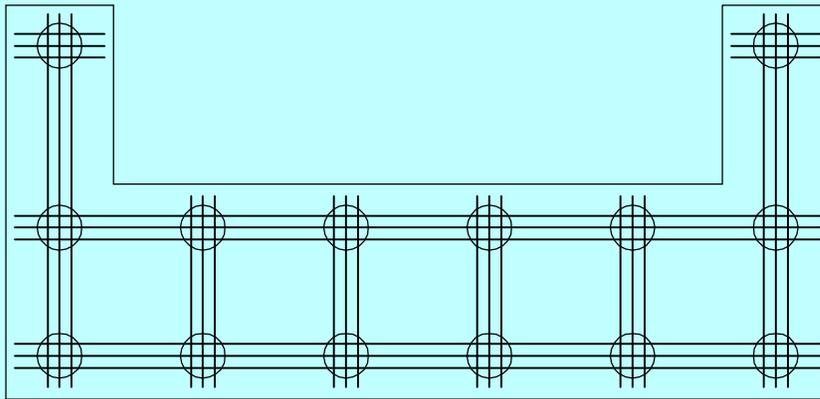
1. All rebars shall be 3-#6 @ 4" c/c.
2. Lower mat of bars shall be 3" (vertical) above top of center line of pile.
3. All piles shall be crossed at least twice.
4. The direction taken by bars shall wherever possible be, the shortest distance between piles.
5. In all cases the total pattern shall be shown on Contract Documents.
6. 0 denotes all piling, cast-in-place, steel H piles, etc. When showing on Contract Documents H piles shall be shown with the normal "H" symbol.
7. In laying out pile plan, if possible, piles shall be positioned to minimize need for diagonal bars.
8. A note in area of rebar pattern shall appear on Contract Documents as follows "Shop plans shall show how rebars are to be tied as well as how they will be held in place above piling while pour is being made."

* FOR OFFICE USE ONLY *

APPROVAL	
<i>L.S. Fisher</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 10/14/82	
REVISIONS	
SHA	FHWA
1-22-01	

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	REBAR MAT PATTERN OVER PILING
STANDARD NO. FND-PF-201	SHEET <u>1</u> OF <u>3</u>

FOUNDATION - PILES



TYPICAL ABUTMENT FOOTING PLANS

Scale: $\frac{3}{16}''=1'-0''$

See notes on Sheet 1 of 3.

* FOR OFFICE USE ONLY *

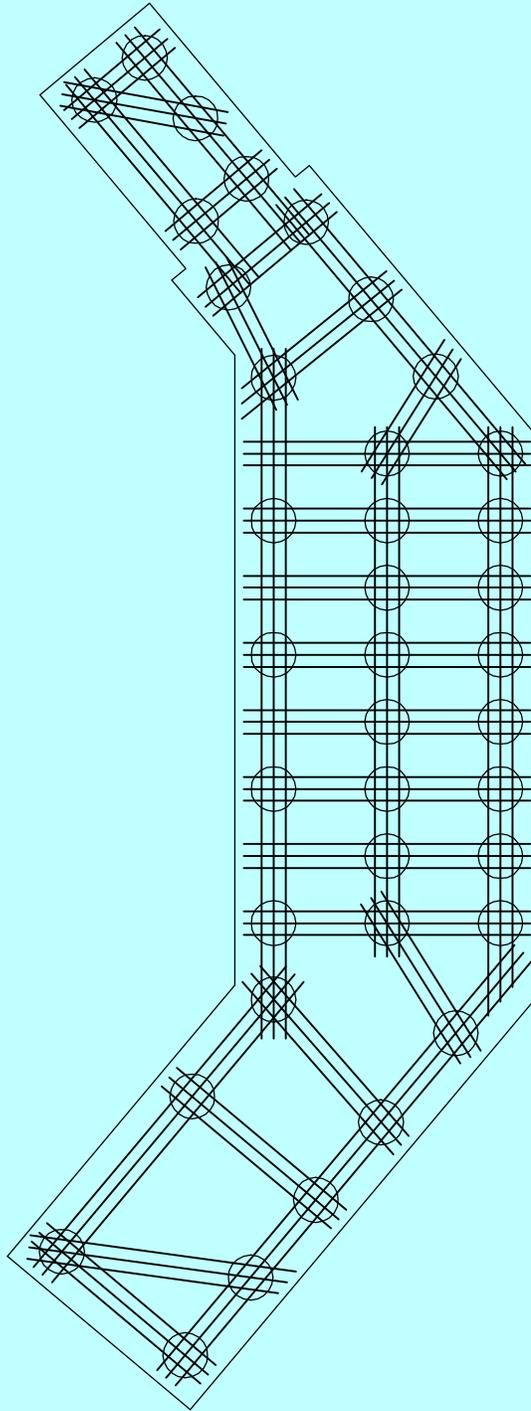
APPROVAL	
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 10/14/82	
REVISIONS	
SHA	FHWA
11-30-87	
3-22-89	
FHWA APPROVAL	1-22-01
DATE:	

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

REBAR MAT PATTERN OVER PILING

STANDARD NO. FND-PF-201

SHEET 2 OF 3



TYPICAL ABUTMENT FOOTING PLAN

Scale: 3/16" = 1'-0"

See notes on Sheet 1 of 3.

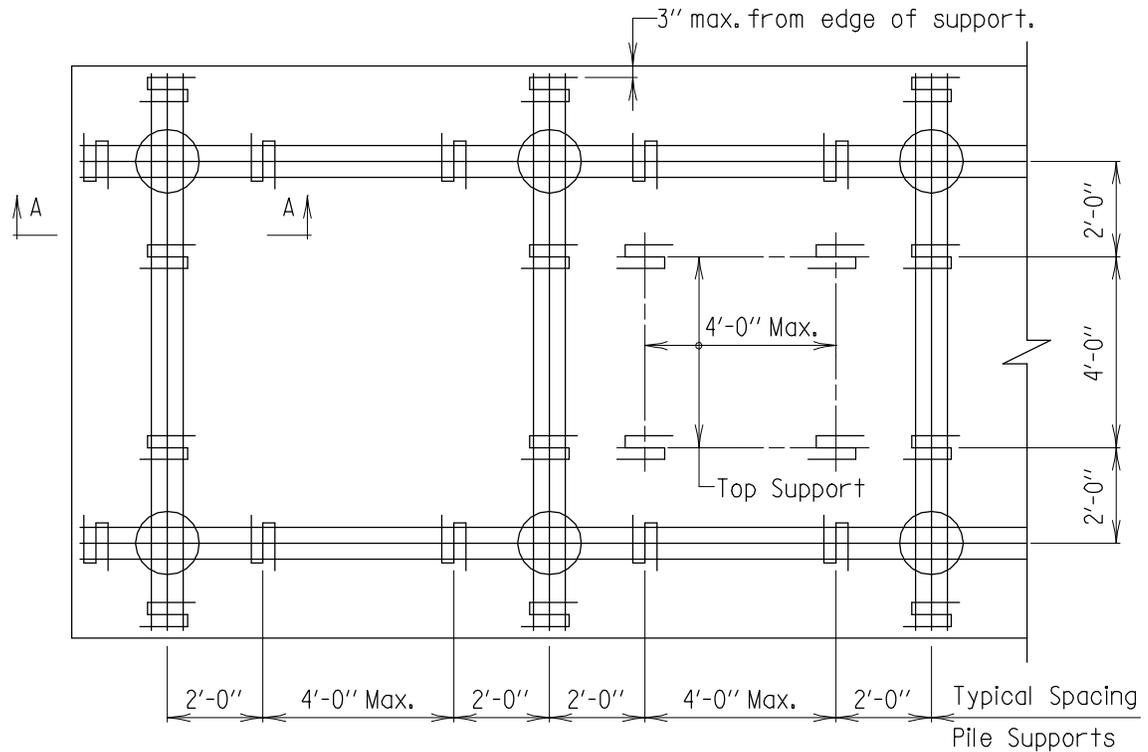
* FOR OFFICE USE ONLY *

APPROVAL	
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES DATE: 10/14/82	
REVISIONS	
SHA	FHWA
1-22-01	

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
REBAR MAT PATTERN OVER PILING
STANDARD NO. FND-PF-201
SHEET <u>3</u> OF <u>3</u>

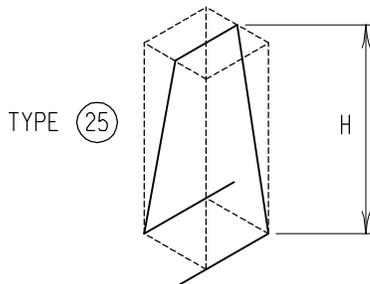
FHWA APPROVAL	
DATE:	

FOUNDATION - PILES



PLAN - PILES (SHOWING MAXIMUM SPACING OF BAR SUPPORTS)

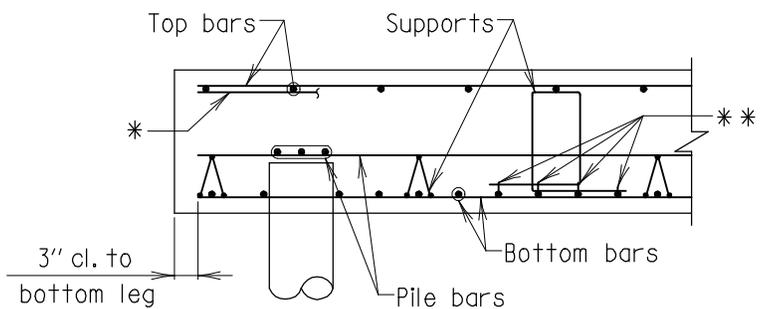
Scale: 1/4" = 1'-0"



ISOMETRIC VIEW

SEE TYPICAL BAR SHEET FOR DIMENSIONS

SIZE	HEIGHT
#4	0'-7" ≤ H ≤ 1'-6"
#5	1'-6" ≤ H ≤ 3'-6"



** Tie each base leg at two intersections to bottom footing bars for stability.

SECTION A-A

Scale: None

DIMENSIONS AND QUANTITIES TO BE SUPPLIED BY CONTRACTOR

* Top bar cannot be dropped to act as a support bar. If support bar is required, separate #5 bars are to be used.

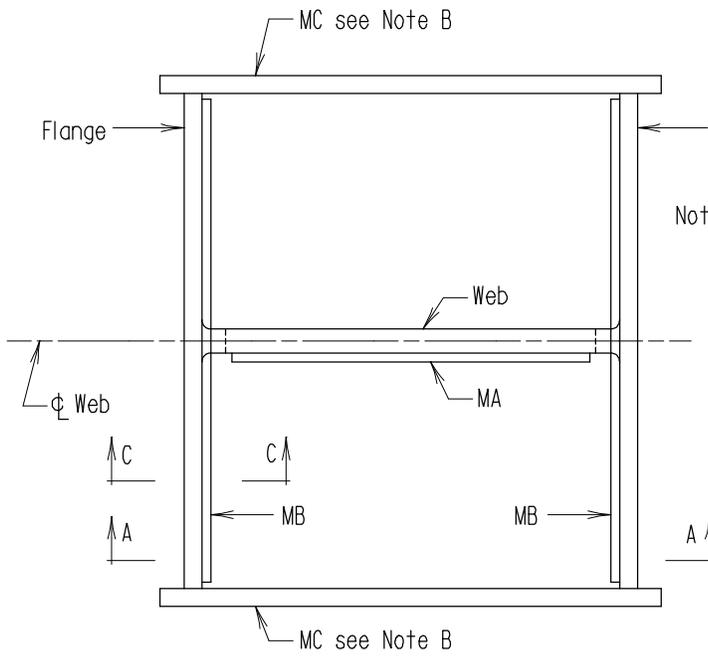
APPROVAL	
<i>L.S. Fisher</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 3/7/91	
REVISIONS	
SHA	FHWA
FHWA APPROVAL	
DATE:	

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

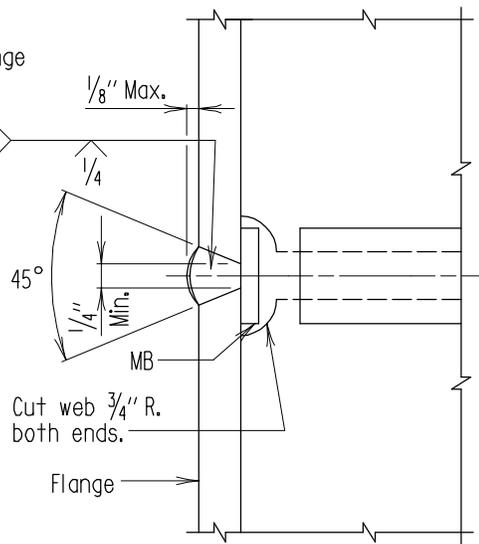
REINFORCING BAR STANDARD
FOOTING SUPPORT SYSTEM

STANDARD NO. FND-PF-202

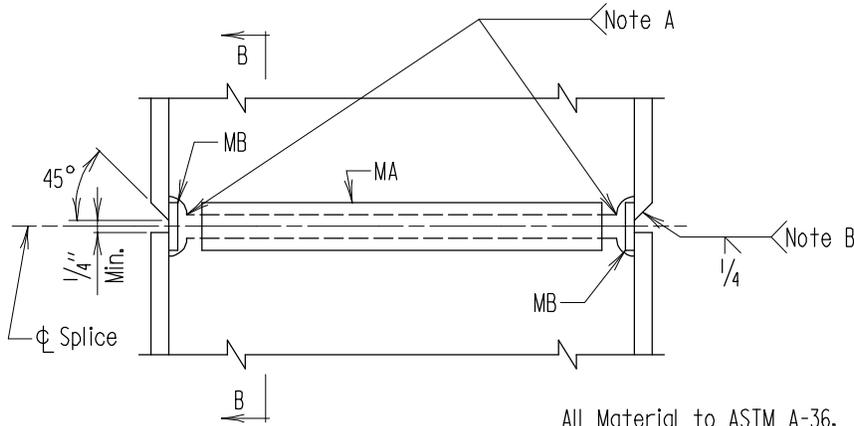
SHEET OF



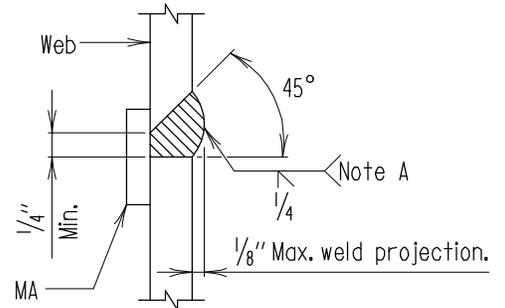
PLAN
Scale: None



SECTION C-C
ALTERNATE FLANGE WELD
Scale: 6" = 1'-0"



SECTION A-A
Scale: None



SECTION B-B
Scale: 6" = 1'-0"

Material Required:

1 Bar MA 1" x 3/16" x 7 1/4"	1 Bar MA 1" x 3/16" x 9 3/8"	1 Bar MA 1" x 3/16" x 1'-0"
For HP 10 Piles 2 Bars MB 1" x 3/16" x 10"	For HP 12 Piles 2 Bars MB 1" x 3/16" x 1'-0"	For HP 14 Piles 2 Bars MB 1" x 3/16" x 1-2 1/2"
2 Bars MC 3" x 3/8" x 11"	2 Bars MC 3" x 3/8" x 1'-1"	2 Bars MC 3" x 3/8" x 1'-3"

Note A:
End of weld to be smooth and flush with web cut, 1/4" min. effective throat.

Note B:
Bar MC to be tack welded to flange at splice to back up end of flange weld, remove MC after weld is completed. End of weld must be smooth and flush with edge of flange. Grind weld smooth with edge of flange if pile is unsupported in weld area such as: in air, water, or soft mud, 1/4" min. effective throat.

Note C:
Let welds cool to air temperature before driving piles.

Note D:
No pile splicing to be allowed on any portion of pile that is to remain exposed or to be above finished groundline in completed structure.

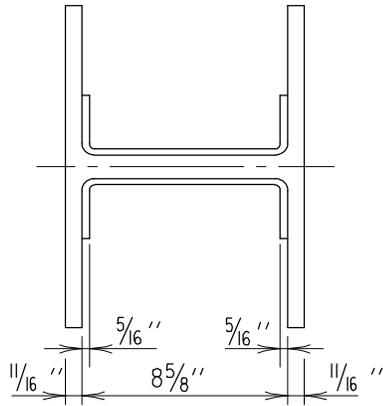
APPROVAL	
L.S. Friedman DIRECTOR OFFICE OF STRUCTURES	
DATE: 6/30/75	
REVISIONS	
SHA	FHWA
9-26-83	6-8-90
4-10-86	6-8-90
FHWA APPROVAL	2-19-92
DATE: 8-24-76	1-22-01

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

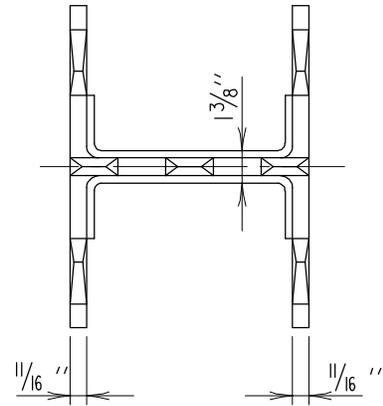
STEEL H PILE SPLICE DETAILS

STANDARD NO. FND-PF-301

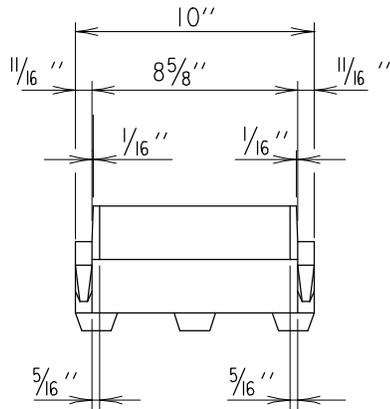
SHEET OF



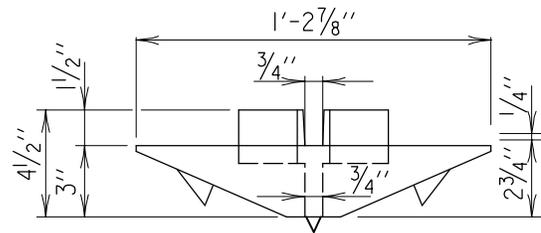
PLAN
Scale: 1 1/2" = 1'-0"



PLAN
Scale: 1 1/2" = 1'-0"



ELEVATION
Scale: 1 1/2" = 1'-0"



SIDE VIEW
Scale: 1 1/2" = 1'-0"

Size of Pile	Size of 45 Bevel	Size of Groove Weld
10 x 42	1/4	5/16
10 x 57	1/4	5/16

Note:

1. Material: Cast Steel A.S.T.M. A27 65/35.
All fillets 3/8".
2. Point to be welded to pile with a continuous single bevel groove weld along outside face of flange. Exterior face of flange to be flame cut beveled at 45° prior to welding.
3. For each shipment of points a foundry certificate verifying material meets the Specifications is required.

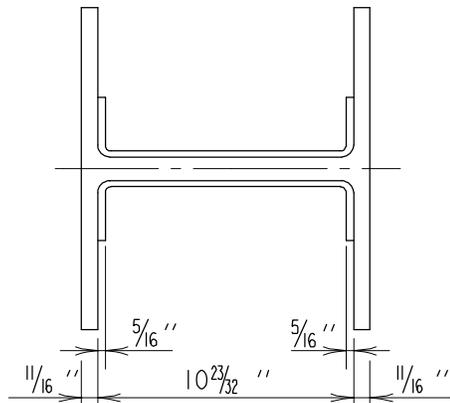
FHWA APPROVAL
DATE: 1-16-80

APPROVAL	
<i>L.S. Freedom</i> DIRECTOR OFFICE OF STRUCTURES DATE: 10/17/79	
REVISIONS	
SHA	FHWA
12-13-79	1-29-80

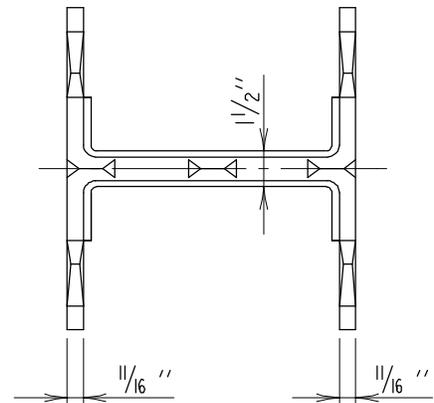
STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

TOOTHED PILE POINT FOR 10" H PILE

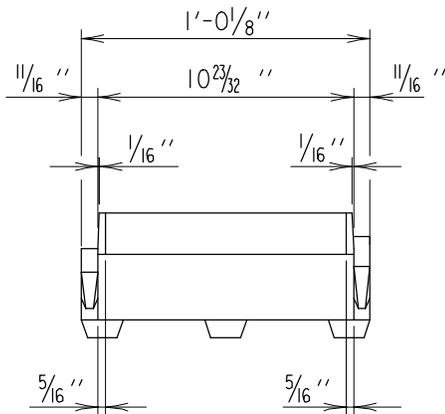
STANDARD NO. FND-PF-302 SHEET 1 OF 1



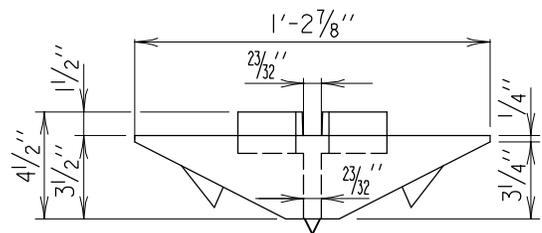
PLAN
Scale: $1\frac{1}{2}'' = 1'-0''$



PLAN
Scale: $1\frac{1}{2}'' = 1'-0''$



ELEVATION
Scale: $1\frac{1}{2}'' = 1'-0''$



SIDE VIEW
Scale: $1\frac{1}{2}'' = 1'-0''$

Size of Pile	Size of 45 Bevel	Size of Groove Weld
12 x 53	1/4	5/16
12 x 74	5/16	5/16

Note:

1. Material: Cast Steel A27 65/35.
All fillets $\frac{3}{8}''$.
2. Point to be welded to pile with a continuous single bevel groove weld along outside face of flange. Exterior face of flange to be flame cut beveled at 45° prior to welding.
3. For each shipment of points a foundry certificate verifying material meets the Specifications is required.

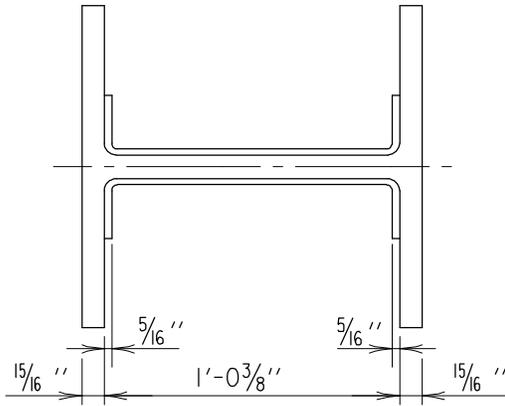
APPROVAL	
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES	
DATE: 10/17/79	
REVISIONS	
SHA	FHWA
12-13-79	1-29-80
4-28-94	
FHWA APPROVAL	
DATE: 1-16-80	

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

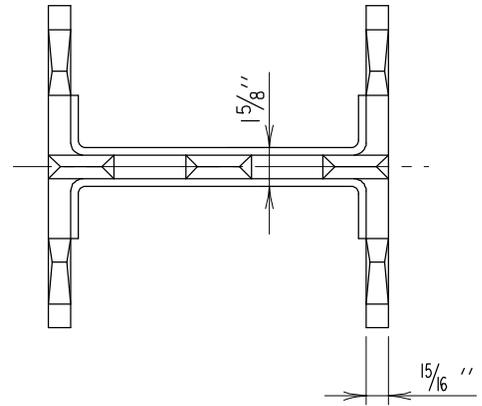
TOOTHED PILE POINT FOR 12" H PILE

STANDARD NO. FND-PF-303

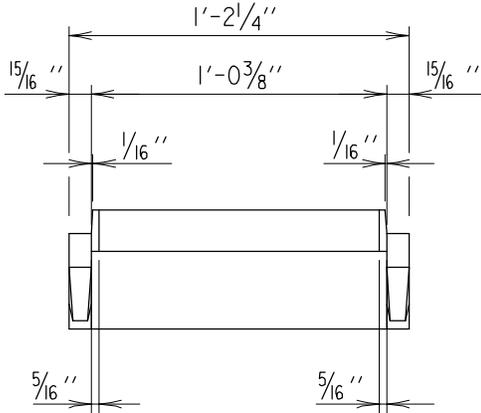
SHEET 1 OF 1



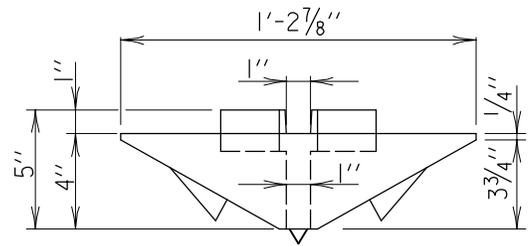
PLAN
Scale: 1 1/2" = 1'-0"



PLAN
Scale: 1 1/2" = 1'-0"



ELEVATION
Scale: 1 1/2" = 1'-0"



SIDE VIEW
Scale: 1 1/2" = 1'-0"

Size of Pile	Size of 45 Bevel	Size of Groove Weld
14 x 73	1/4	5/16
14 x 89	5/16	5/16
14 x 102	3/8	3/8
14 x 117	3/8	7/16

Note:

- Material: Cast Steel A.S.T.M. A27 65/35. All fillets 3/8".
- Point to be welded to pile with a continuous single bevel groove weld along outside face of flange. Exterior face of flange to be flame cut beveled at 45° prior to welding.
- For each shipment of points a foundry certificate verifying material meets the Specifications is required.

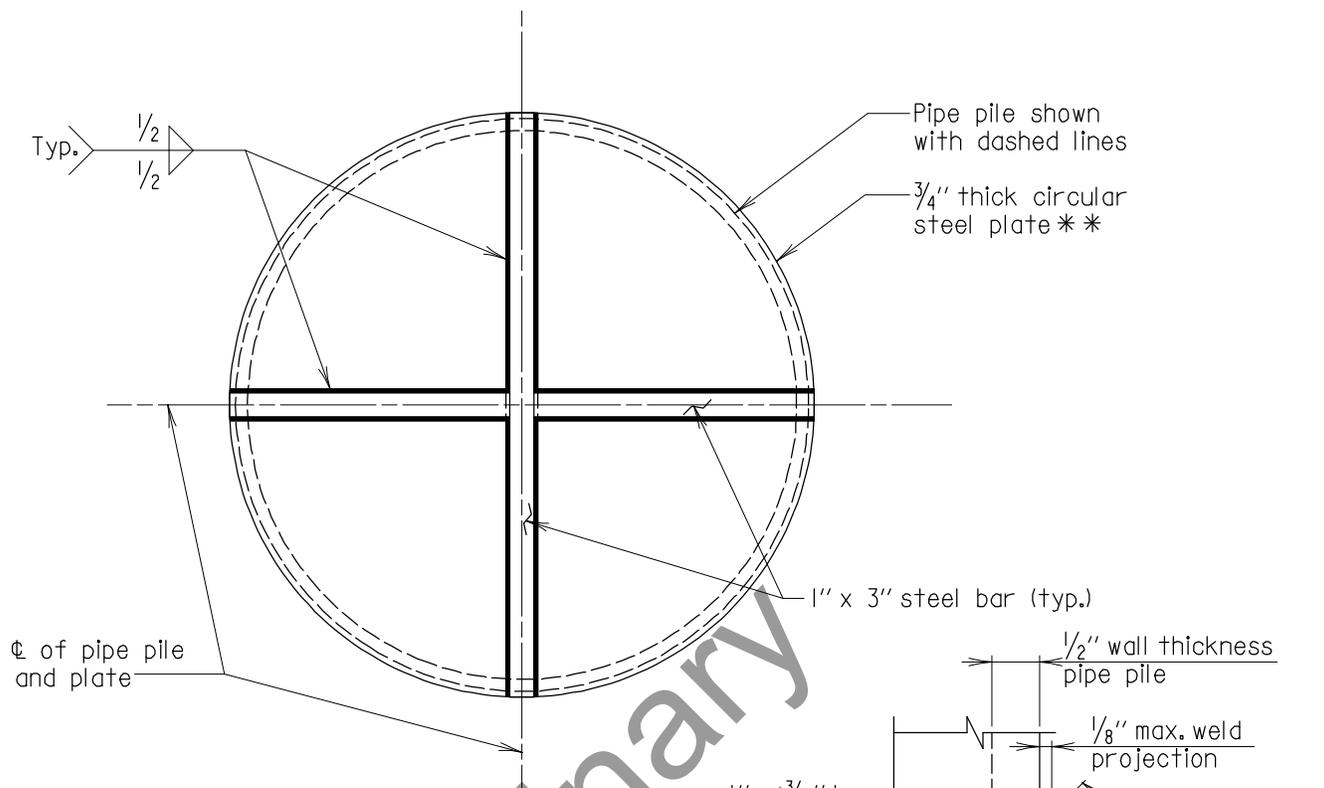
APPROVAL	
<i>L.S. Freedom</i> DIRECTOR OFFICE OF STRUCTURES DATE: 11/14/79	
REVISIONS	
SHA	FHWA
12-13-79	1-29-80

STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

TOOTHED PILE POINT FOR 14" H PILE

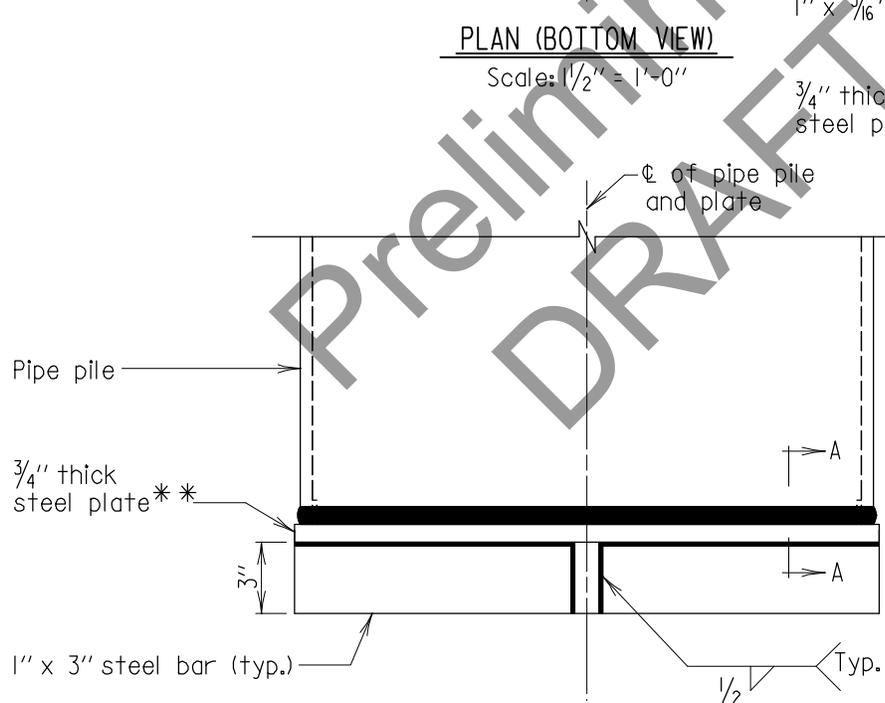
STANDARD NO. FND-PF-304 SHEET OF

FHWA APPROVAL
DATE: 1-16-80



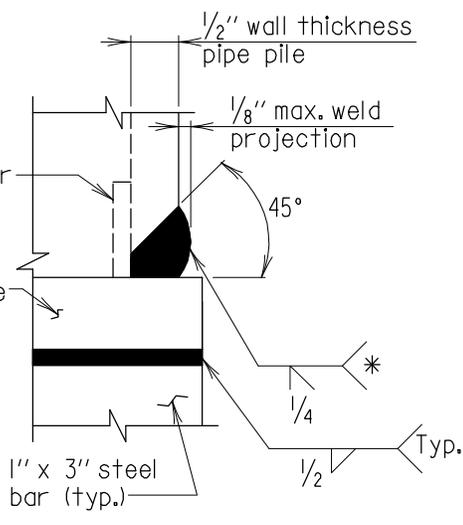
PLAN (BOTTOM VIEW)

Scale: $\frac{1}{2}'' = 1'-0''$



ELEVATION

Scale: $\frac{1}{2}'' = 1'-0''$



SECTION A-A

Scale: $\frac{1}{2}'' = 1'-0''$

* End of weld to be smooth and flush with pipe pile wall cut, $\frac{1}{4}''$ min. effective throat.

Notes:

1. Plate to be welded to pile with a continuous single bevel groove weld along outside face of pile.
2. Exterior face of pile to be flame cut beveled at 45°, prior to welding.
- **3. Circular steel plate shall have a diameter that is $\frac{1}{2}''$ larger than outside diameter of steel pipe pile.

* PRELIMINARY DRAFT *

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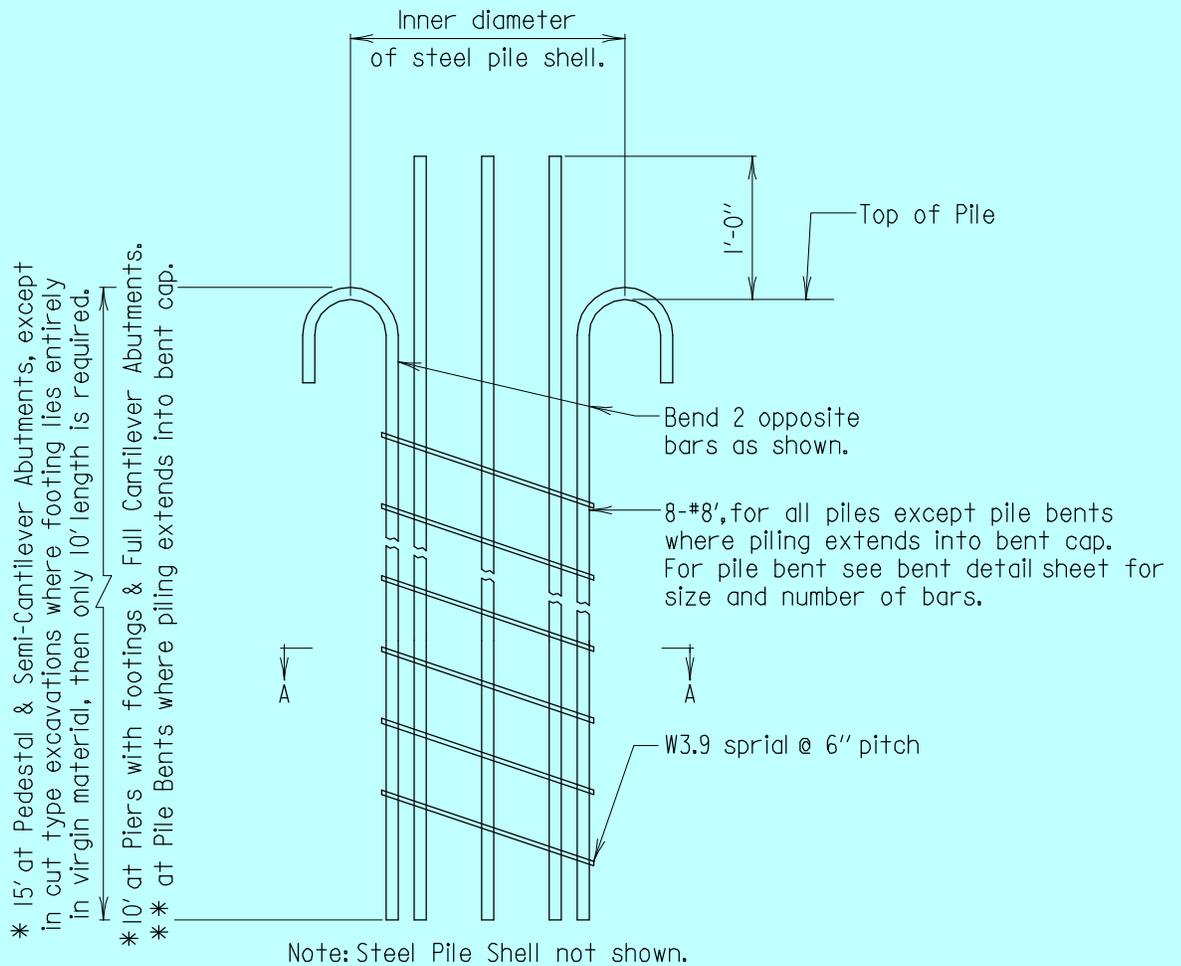
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PILE POINT FOR PIPE PILES

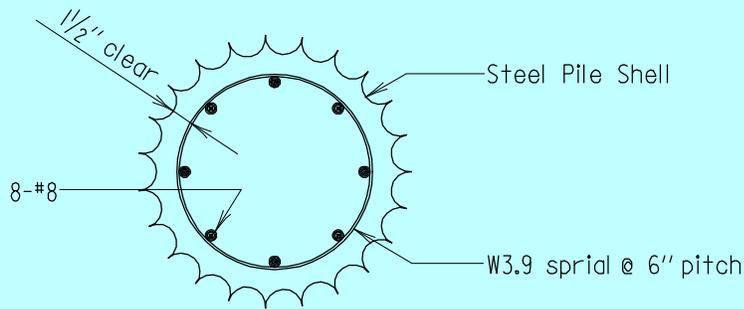
NO. FND-PF-401

SHEET OF

FOUNDATION - PILES



ELEVATION
Scale:None



SECTION A-A
Scale:None

Note:

- 1.* Unless otherwise indicated on substructure contract drawing.
- 2.**Full height of pile above finished groundline plus 10' unless otherwise indicated on substructure contract drawings.
- 3.Unless otherwise indicated on other Contract Plans or in the Special Provisions the Steel Pile Shell shall be a minimum #5 gauge. This will apply to pile shells with or without deformations.
- 4.Cage required for all pile shells with or without deformations.
- 5.All materials and dimensions shown are minimums. Engineer shall design.

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6-16-92	
10-27-92	
FHWA APPROVAL	1-22-01
DATE: 12-19-79	10-9-07

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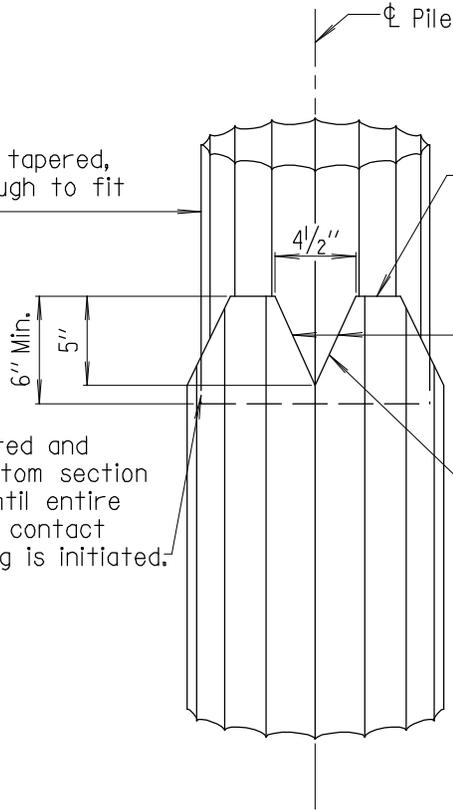


DETAILS
FOR CAST-IN-PLACE CONCRETE PILES

STANDARD NO. FND-PF-501

SHEET OF

If "male end" is not factory tapered, it shall be tapered just enough to fit into lower section.



Continuous fillet weld, minimum size equal to thickness of pile shell.

Burn four equally spaced slots in the lower section before inserting extension.

After pile is aligned, tack weld at all four slots prior to commencing full weld operations.

Top extension shall be inserted and driven if necessary into bottom section (after "V" cutting is done) until entire area of weldment is in tight contact before alignment tack welding is initiated.

ELEVATION

Scale: None

Note:

No pile splicing to be allowed on any portion of pile that is to remain exposed in completed structure.

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SPLICE FOR CAST-IN-PLACE
CONCRETE PILE SHELL

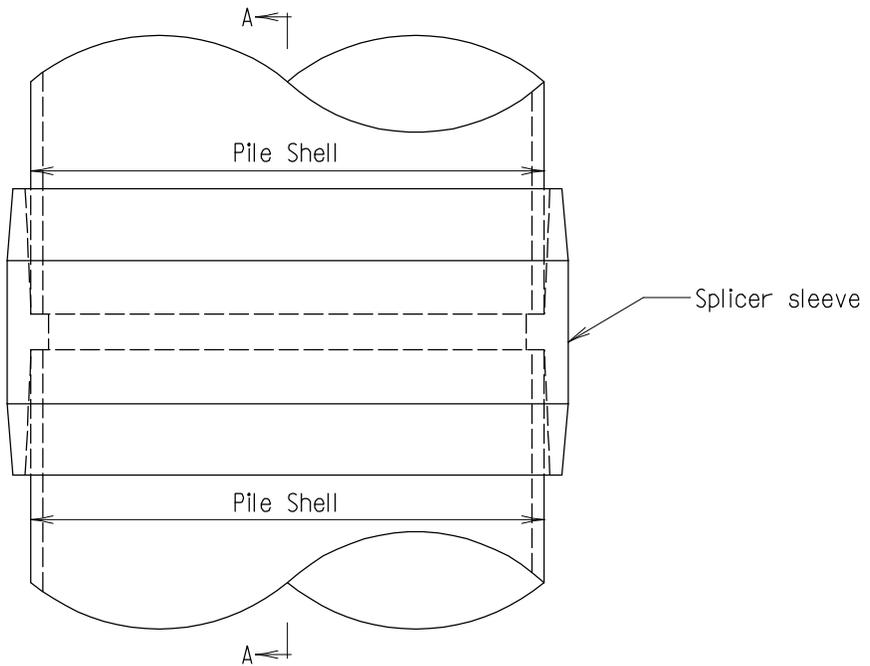
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STANDARD NO. FND-PF-502

SHEET 1 OF 1

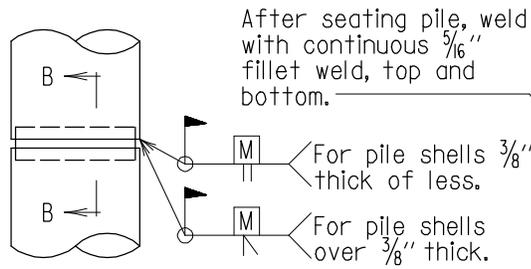
OLD NO. BR-FD(0.02)-75-16

FOUNDATION - PILES



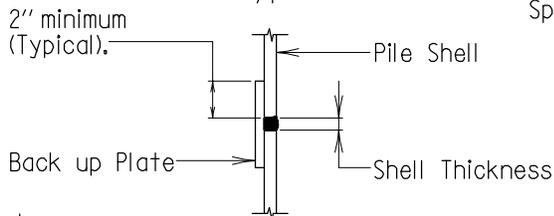
ELEVATION OF SPLICE
(USING SPLICER SLEEVE)

Scale: 3" = 1'-0"



ELEVATION OF SPLICE
(USING ALL WELDED ALTERNATE)

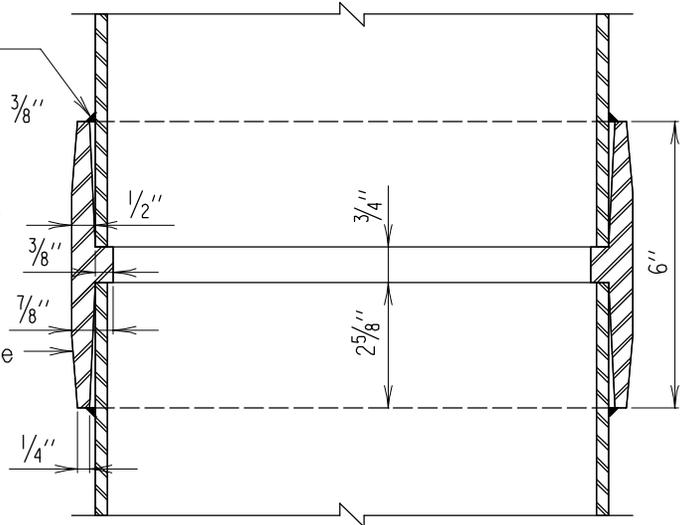
Scale: 3/4" = 1'-0"



Note:
Back-up plate to be cut from same pile size as is being spliced. Cut and bend to fit inside diameter of pile.

SECTION B-B

Scale: None



SECTION A-A

Scale: 3" = 1'-0"

- Notes:
1. No pile splicing to be allowed on any portion of pile that is to remain exposed in completed structure.
 2. Splicer sleeve material shall be steel conforming to ASTM A-36.
 3. Contractor has the option of using either the "Splicer Sleeve" or "All Welded" alternates.

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11-14-85	6-8-90
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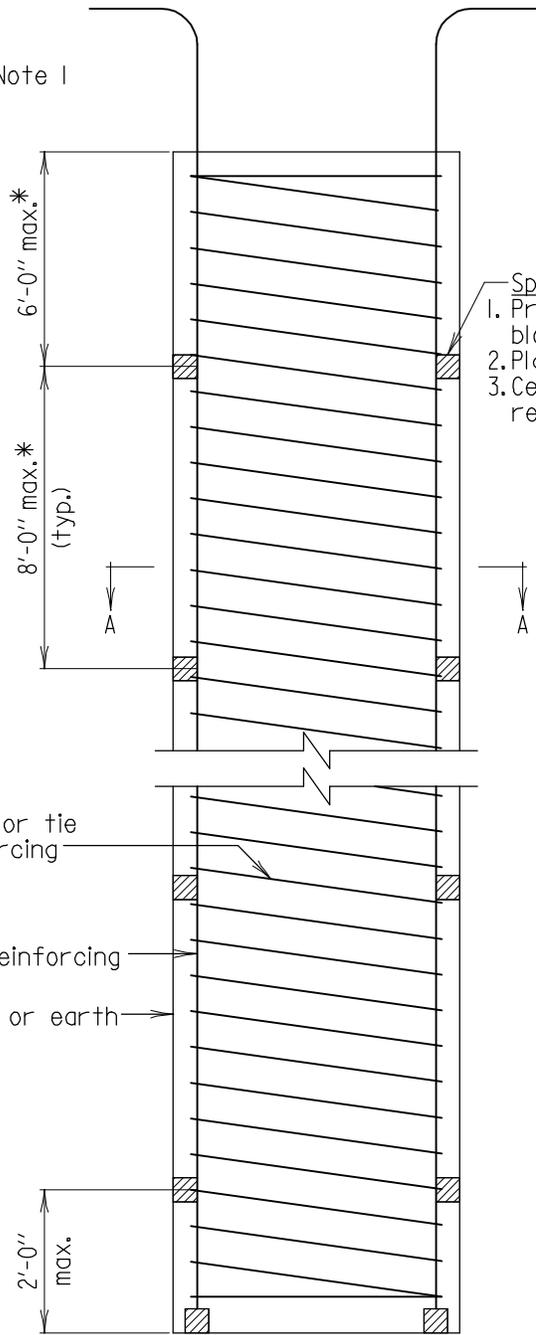
SPLICE FOR CAST-IN-PLACE
CONCRETE PIPE PILE SHELL

STANDARD NO. FND-PF-503

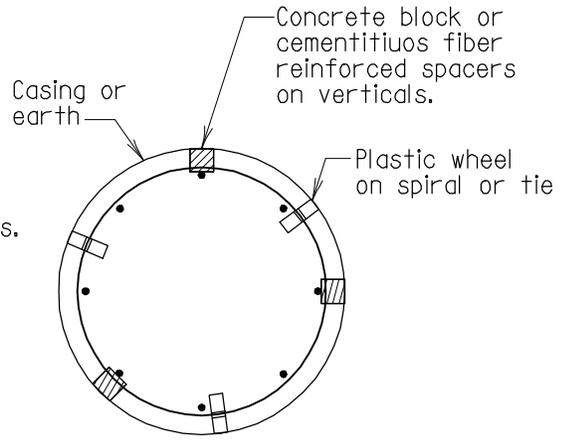
SHEET 1 OF 1

OLD NO. BR-FD(0.11)-83-152

* See Note 1



- Spacer types:
1. Precast concrete block
 2. Plastic wheel
 3. Cementitious fiber reinforced spacers.



SECTION A-A
Scale: $\frac{3}{8}'' = 1'-0''$

Spiral or tie reinforcing
Main reinforcing
Casing or earth

Notes:

1. The Contractor has the option of using any of the spacers shown for the reinforcement cage. The spacing of the spacers for proprietary items shall be as recommended by the manufacturer.
2. The pitch of spiral reinforcement must be considered for some wheel type spacers.
3. Concrete spacer blocks to be tied to main reinforcing with a double tie of #16 tie wire or equivalent.
4. For size and number of main reinforcing steel and size of spiral or tie reinforcing steel see other details elsewhere.
5. Use 3 spacers per horizontal plane for caissons less than 36" in diameter. Use 4 spacers per horizontal plane for caissons 36" in diameter and greater or as recommended by the manufacturer of the proprietary items.

PILE OR CAISSON
Scale: $\frac{3}{8}'' = 1'-0''$

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CAISSON/PILE REBAR CAGE
CLEARANCE SPACING DEVICES

STANDARD NO. FND-PF-504

SHEET OF

FOUNDATIONS

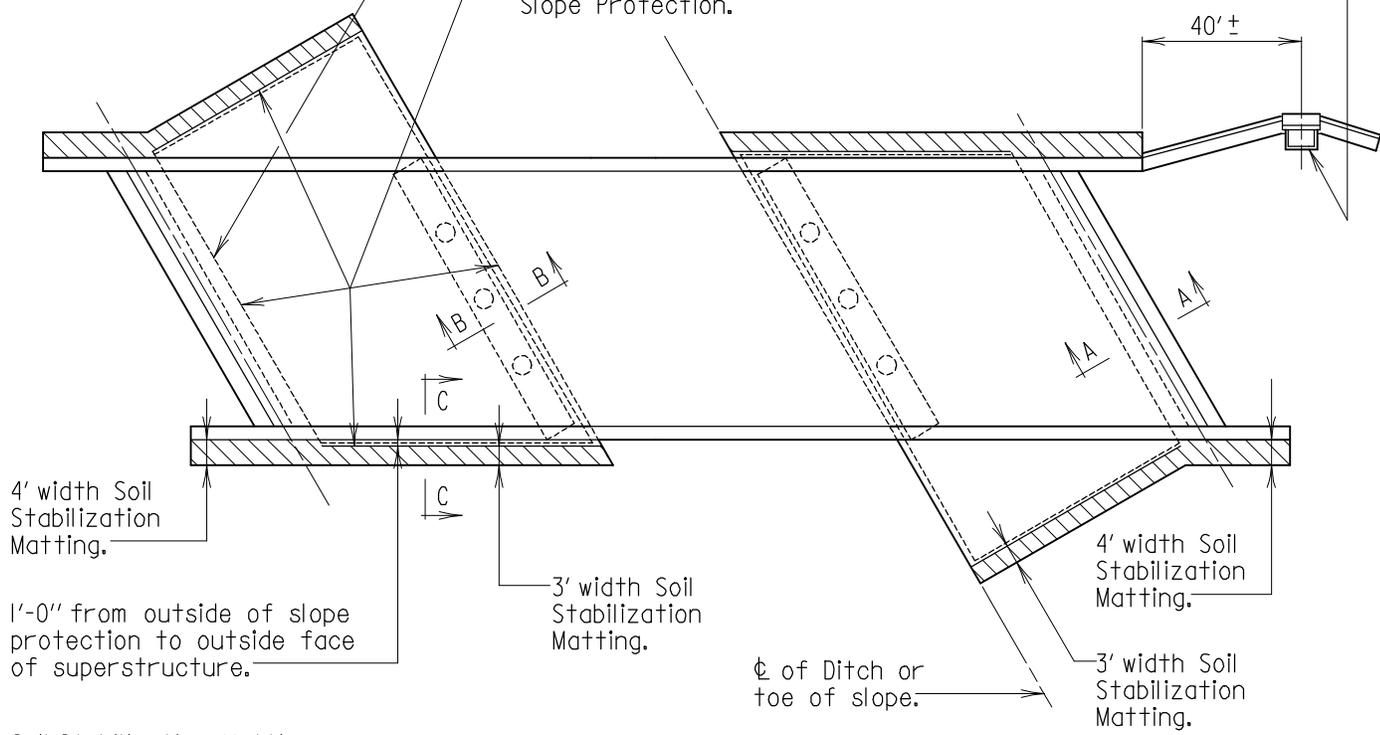
SLOPE PROTECTION

(FND-SP)

Front face of Abutment.

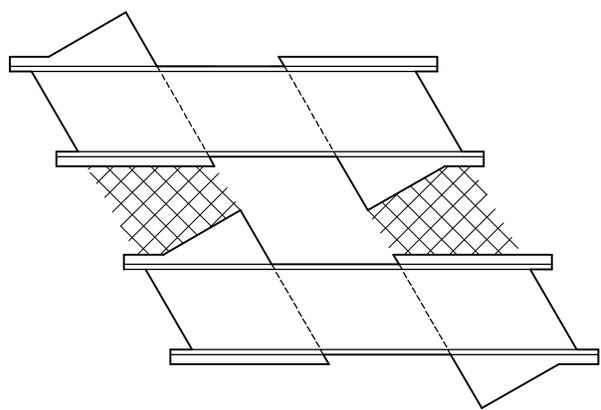
Limits of measurement for basis of Payment for Slope Protection.

Place Inlet on all downhill end(s) where crown is forcing water to that location.



Soil Stabilization Matting shown hatched.

PLAN
Scale: None



SKETCH-PLAN

Note:
On dual bridges where perpendicular distance between bridge faces is less than 30' or on skewed dual bridges if where the unprotected area for both ends of the bridges (hatched areas in sketch-Plan) were added is less than 200 sq. ft., then slope protection is to be continuous thru median area.

- Notes:
1. For Sections A-A, B-B, and C-C see sheets 2 thru 6 of 6.
 2. If limits for slope protection are shown on Contract Drawings, then those limits take precedent over what is shown on this sheet.

Slanted lettering indicate notes "For Office Use Only".

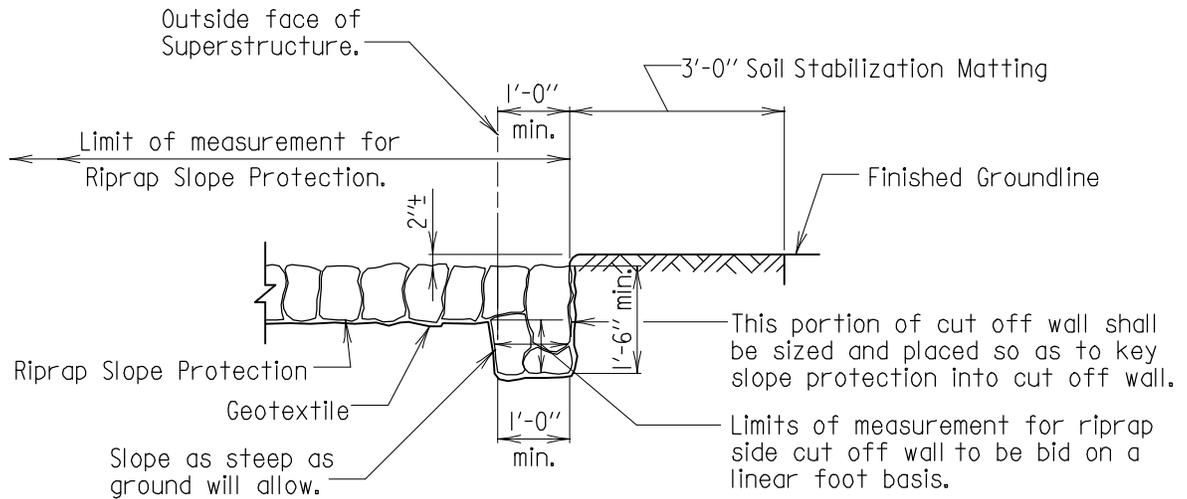
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DATE: 4/15/78	
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9-25-81	11-25-85
FHWA APPROVAL DATE: 10-17-78	3-21-89
	11-15-95

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SLOPE PROTECTION FOR BRIDGES
CARRYING ROAD OVER ROAD OR RAILROADS

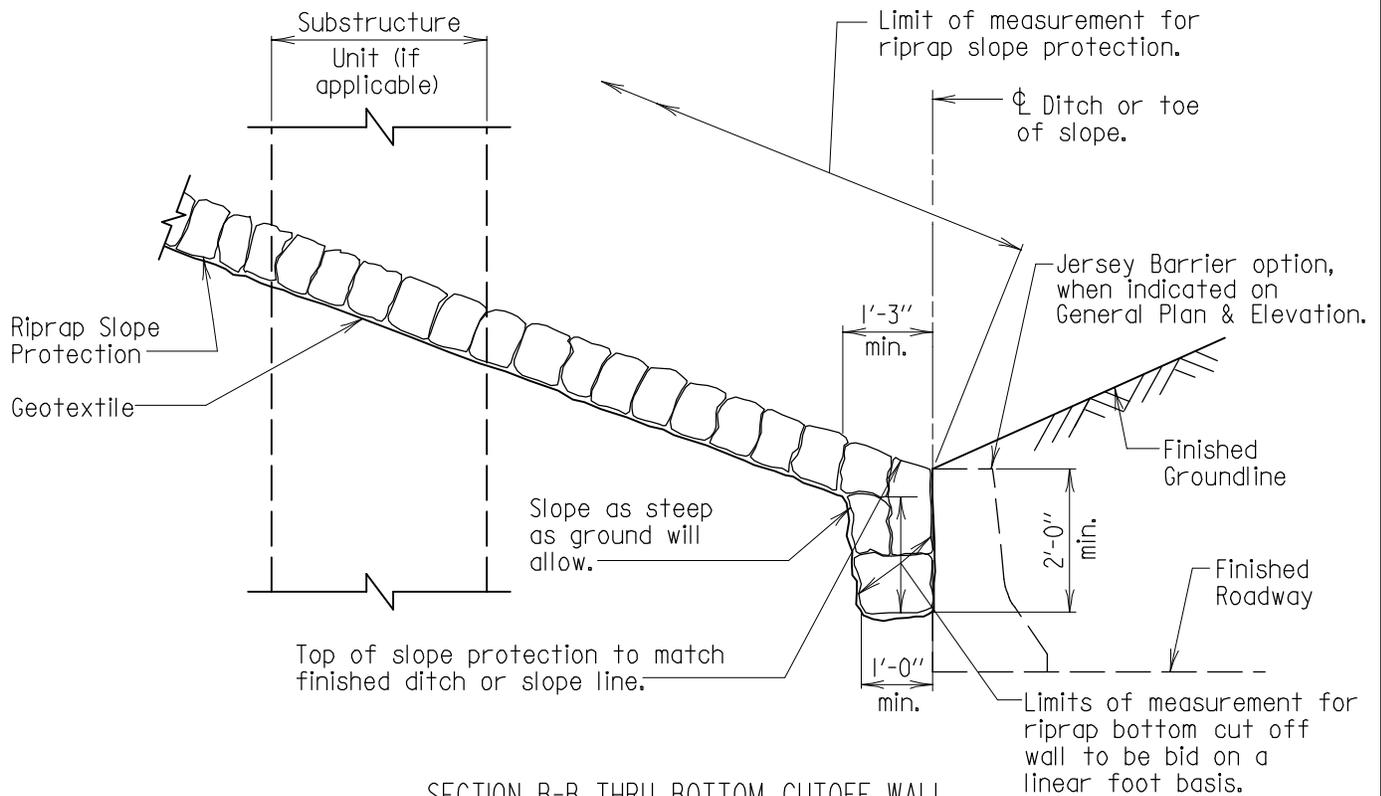
STANDARD NO. FND-SP-101

SHEET 1 OF 6



SECTION C-C THRU SIDE OF CUTOFF WALL

Scale: $\frac{3}{8}$ " = 1'-0"



SECTION B-B THRU BOTTOM CUTOFF WALL

Scale: $\frac{3}{8}$ " = 1'-0"

Note:
If a barrier configuration is used at bottom of slope, the bottom cut off wall shall be eliminated.

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11-15-95	
FHWA APPROVAL	
DATE: 11-29-85	6-29-05

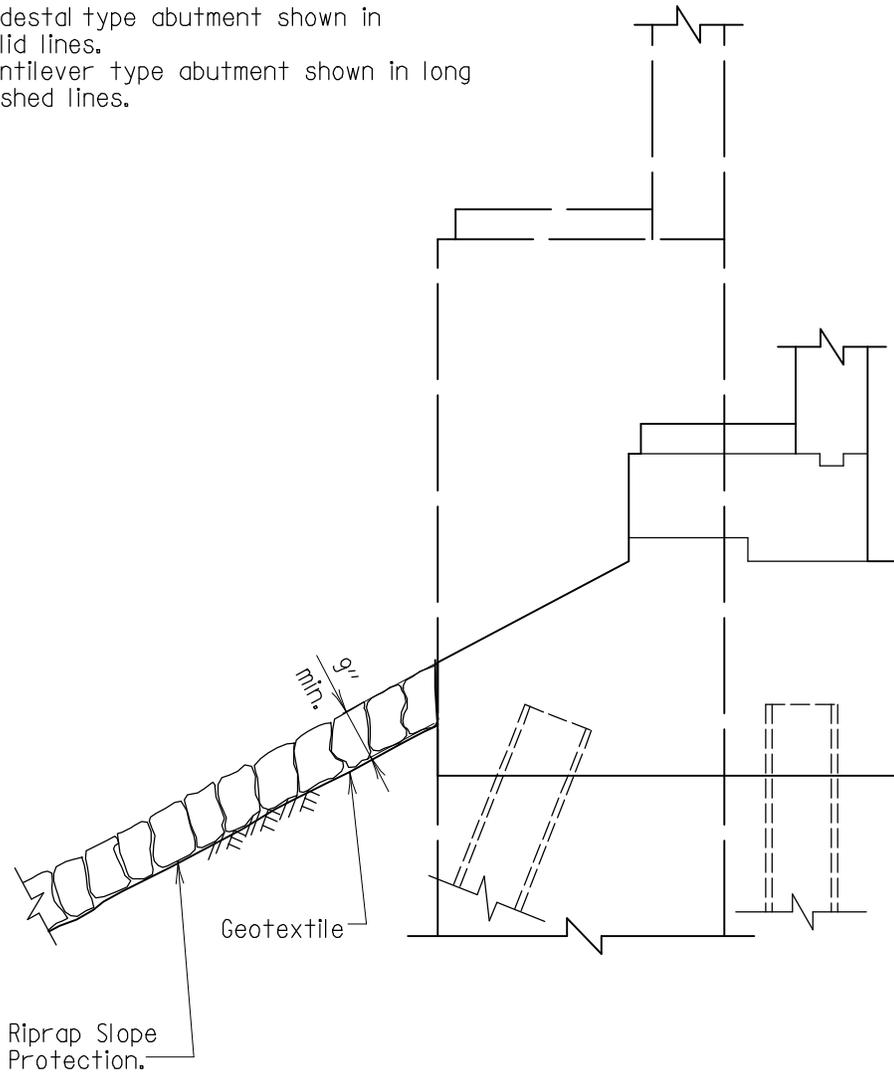
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RIPRAP SLOPE PROTECTION FOR BRIDGES
CARRYING ROAD OVER ROAD OR RAILROAD

STANDARD NO. FND-SP-101

SHEET 2 OF 6

Note:
 Pedestal type abutment shown in solid lines.
 Cantilever type abutment shown in long dashed lines.



SECTION A-A THRU ABUTMENT

Scale: $\frac{3}{8}'' = 1'-0''$

Notes:

1. Bottom cut off wall may be eliminated if slope protection can be founded in rock.
2. All material for riprap slope protection shall be Class I conforming to 901.02.
3. Refer to Section 312 for other requirements.

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DATE: 6-8-90	9-16-11

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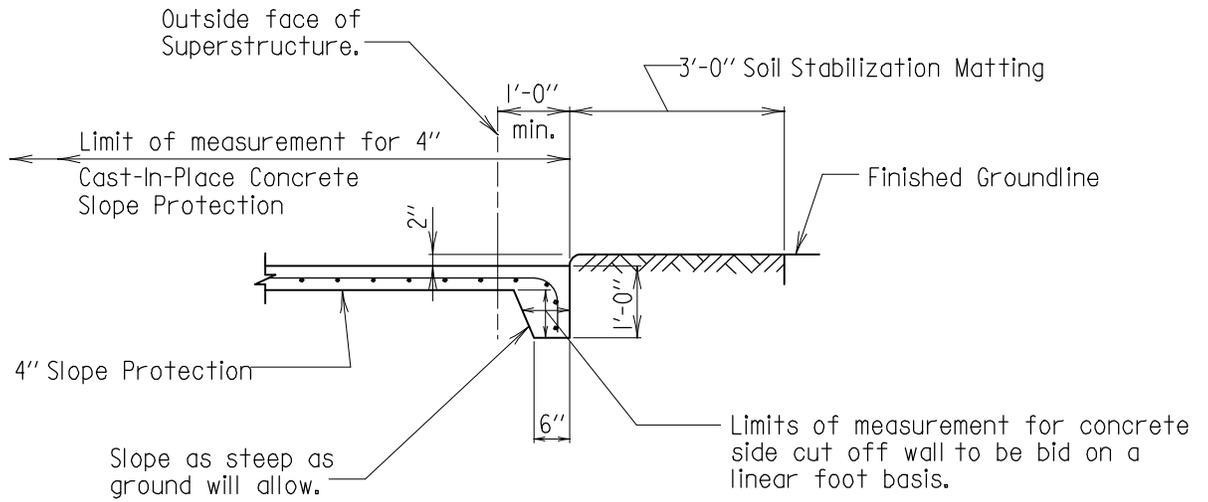
RIPRAP SLOPE PROTECTION FOR BRIDGES
 CARRYING ROAD OVER ROAD OR RAILROAD

STANDARD NO. FND-SP-101

SHEET 3 OF 6

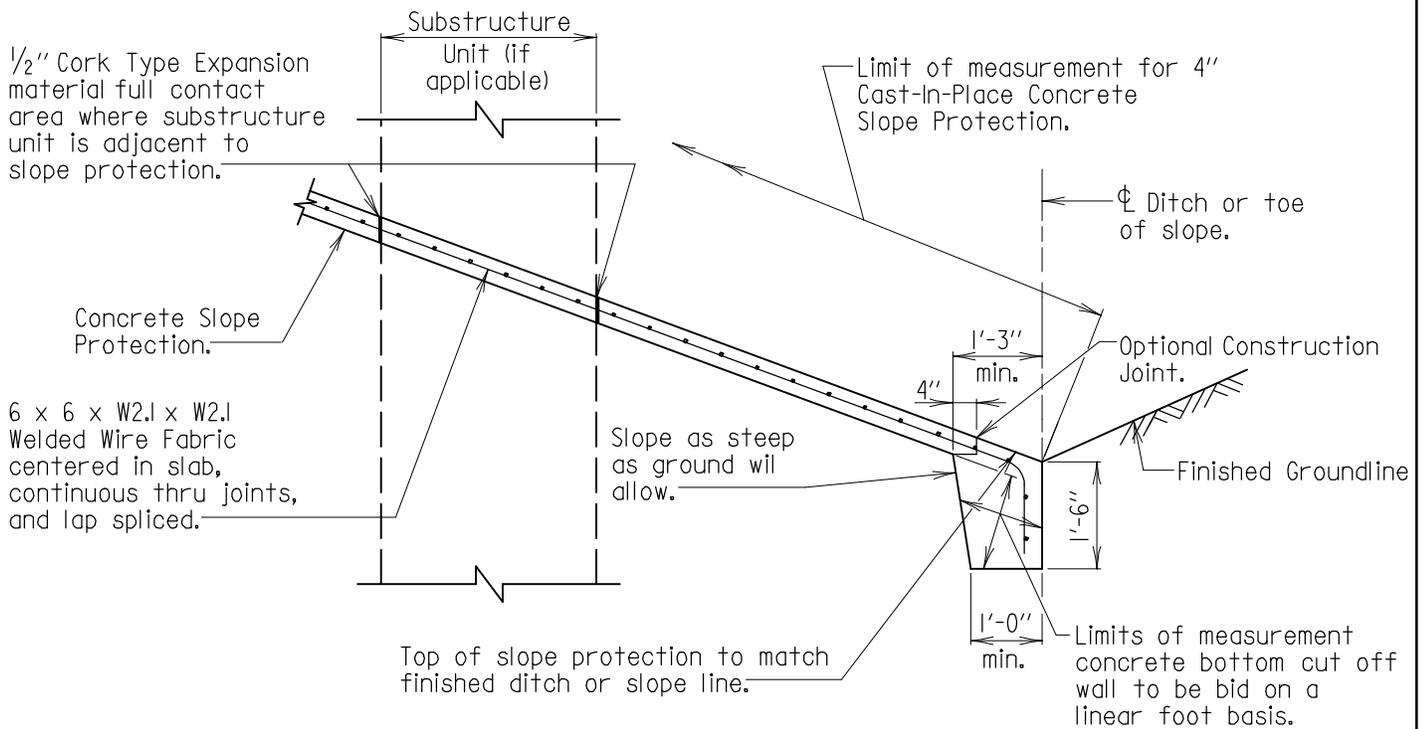
OLD NO. M(6.02)-78-75

FOUNDATION - SLOPE



SECTION C-C THRU SIDE OF CUTOFF WALL

Scale: $\frac{3}{8}''=1'-0''$



SECTION B-B BOTTOM OF CUTOFF WALL

Scale: $\frac{3}{8}''=1'-0''$

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3-21-89	6-8-90
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DATE: 10-17-78	1-22-01

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CONCRETE SLOPE PROTECTION FOR BRIDGES
CARRYING ROAD OVER ROAD OR RAILROAD

STANDARD NO. FND-SP-101

SHEET 4 OF 6

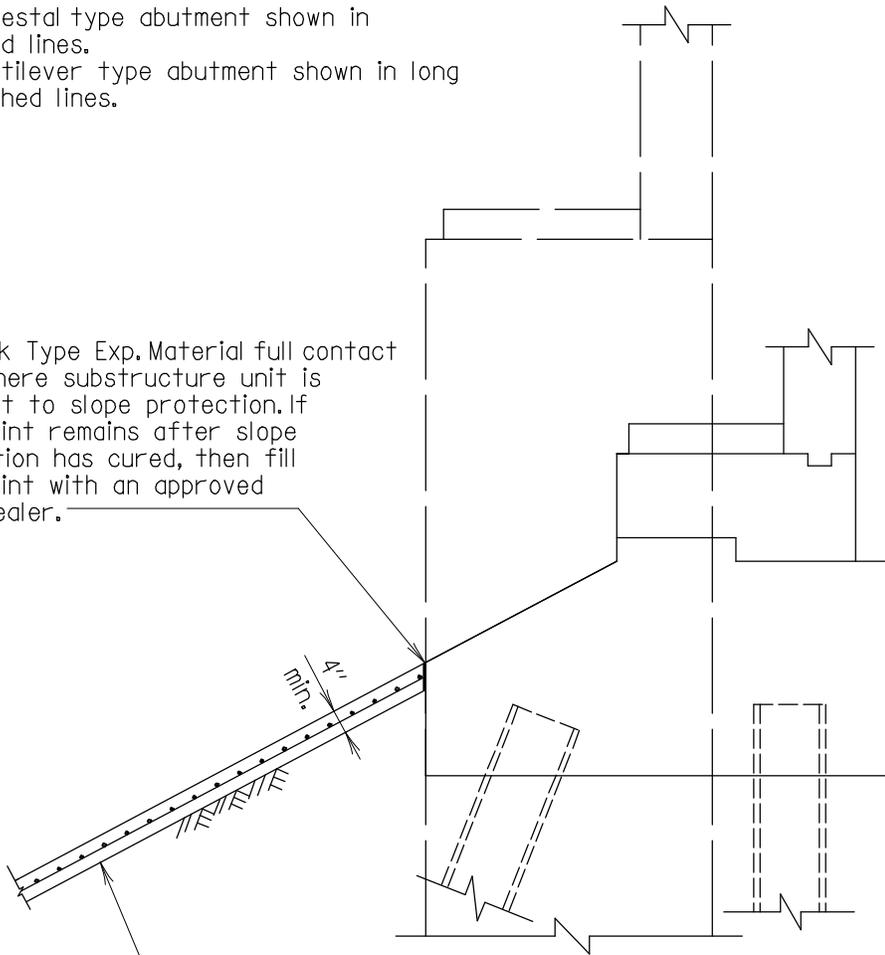
OLD NO. M(6.02)-78-75

FOUNDATION - SLOPE

Note:
 Pedestal type abutment shown in solid lines.
 Cantilever type abutment shown in long dashed lines.

1/2" Cork Type Exp. Material full contact area where substructure unit is adjacent to slope protection. If open joint remains after slope protection has cured, then fill open joint with an approved joint sealer.

Concrete Slope Protection



SECTION A-A THRU ABUTMENT

Scale: 3/8" = 1'-0"

Notes:

1. Bottom cut off wall may be eliminated if slope protection can be founded in rock.
2. Refer to Section 310 for other requirements.

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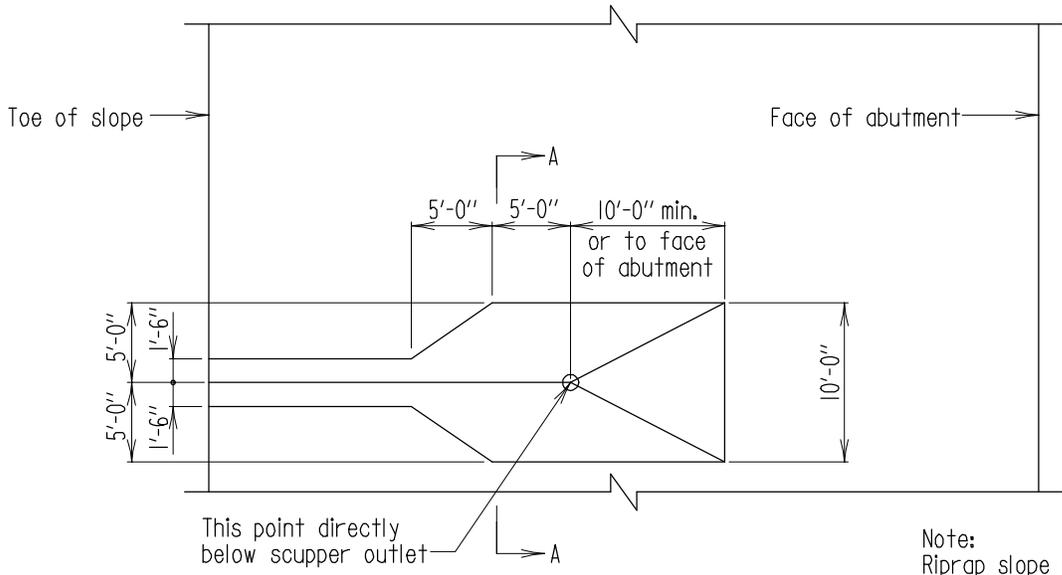
CONCRETE SLOPE PROTECTION FOR BRIDGES
 CARRYING ROAD OVER ROAD OR RAILROAD

STANDARD NO. FND-SP-101

SHEET 5 OF 6

OLD NO. M(6.02)-78-75

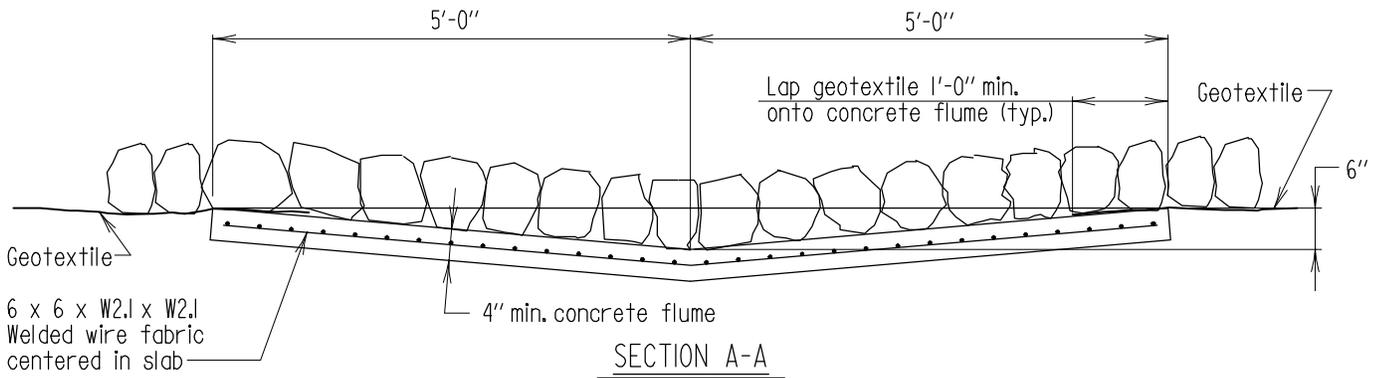
FOUNDATION - SLOPE



This point directly below scupper outlet

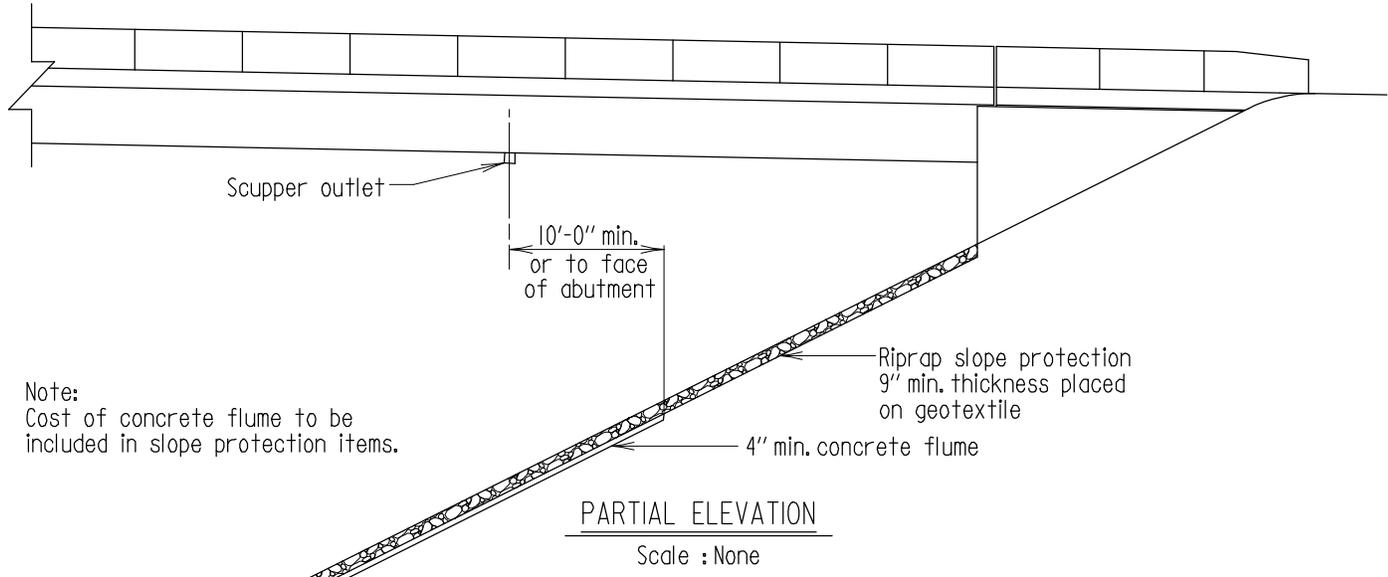
Note:
Riprap slope protection not shown.

PLAN
Scale : None



6 x 6 x W2.1 x W2.1
Welded wire fabric
centered in slab

SECTION A-A
Scale : 1/2" = 1'-0"



Note:
Cost of concrete flume to be included in slope protection items.

PARTIAL ELEVATION
Scale : None

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CONCRETE SPLASH BLOCK ON END SLOPE WITH RIPRAP SLOPE PROTECTION	
STANDARD NO. FND-SP-101	SHEET 6 OF 6

GENERAL NOTES

SELECTION OF THE RIPRAP D50 SIZE AND BLANKET THICKNESS:

The FHWA equations from HEC-23, Bridge Scour and Stream Instability Countermeasures (Design Guideline 8, Rock Riprap at Abutments and Piers) should be used to compute the minimum required D50 size of riprap. This value is to be compared with the D50 size of riprap in the table below to select the appropriate riprap Class and blanket thickness. As noted previously, use of Class I riprap is not recommended except for certain conditions, see sheet 5 of this standard.

RIPRAP CLASS	D50 MINIMUM SIZE (INCHES)	APPROXIMATE D50 WEIGHT (POUNDS)	MINIMUM BLANKET THICKNESS (INCHES)*
I	9.5	40	19
II	16	200	32
III	23	600	46

*These dimensions apply to the upper blanket section only, not the toe section.

DESIGN OF THE TOE SECTION:

A stable riprap toe is the most important feature in the design of riprap abutment protection installations. Guidance on the design of the toe section is provided on sheet 2 of this standard. The following criteria serve to establish the design for the riprap toe:

1. Design the riprap toe extend below the depth of contraction scour in the scour cross-section (see sheet 2 of this standard).
2. The riprap toe should be at least 6 feet thick. (A lesser toe thickness may be appropriate under certain field conditions as depicted on sheet 45 of this standard).
3. The top width of the riprap toe is typically 12 feet or more in order to fit the riprap geometry to the ground conditions.
4. An aggregate or geotextile filter cloth is normally used with the riprap installation.

RIPRAP SPECIFICATIONS:

The following riprap specification are set forth in the July 2008 Edition of the SHA Standard Specifications for Construction and Materials:

Construction: Section 312, Riprap Slope and Channel Protection.

Materials: Section 901.01, Aggregate ; 901.02 Stone for Riprap; 921.09 Geotextile.

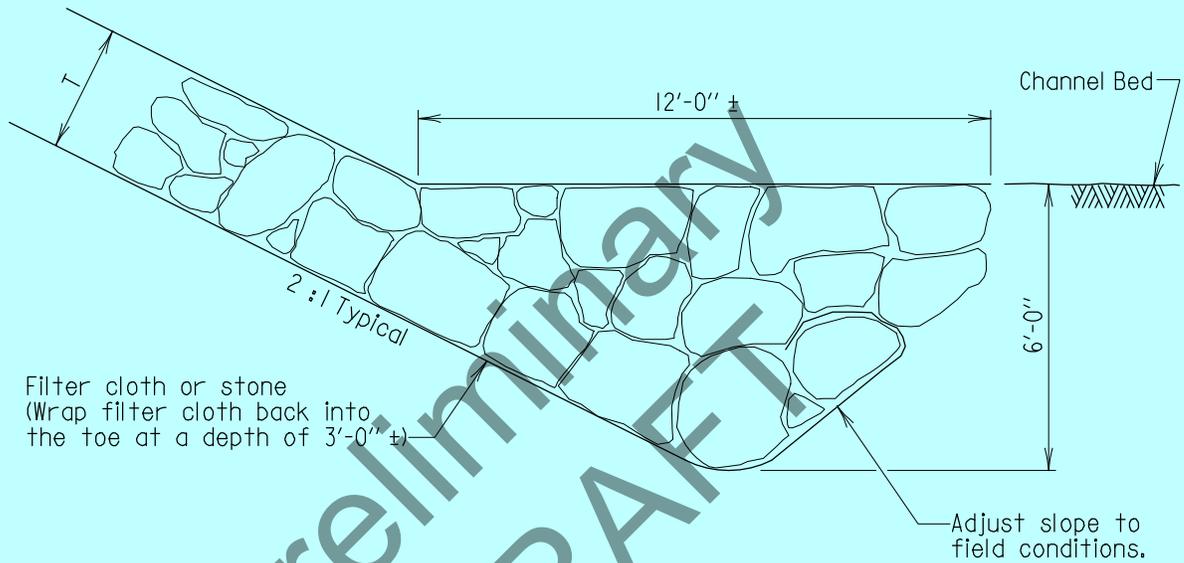
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TYPICAL RIPRAP INSTALLATIONS AT PIERS AND ABUTMENTS GENERAL NOTES	
NO. FND-SP-201	SHEET <u>1</u> OF <u>6</u>

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T = 32" (min.) Class 2
 T = 46" (min.) Class 3



TYPICAL RIPRAP BLANKET AND TOE DETAIL

Scale: 1/4" = 1'-0"

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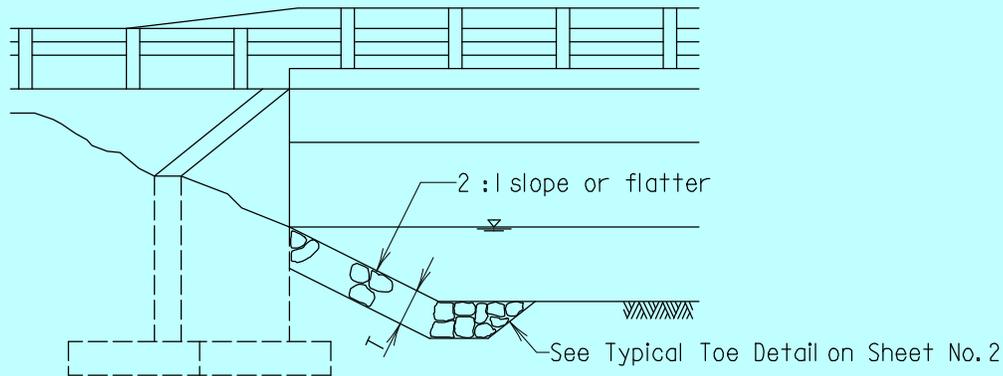
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TYPICAL RIPRAP INSTALLATIONS
 AT PIERS AND ABUTMENTS
 DETAILS

NO. FND-SP-201

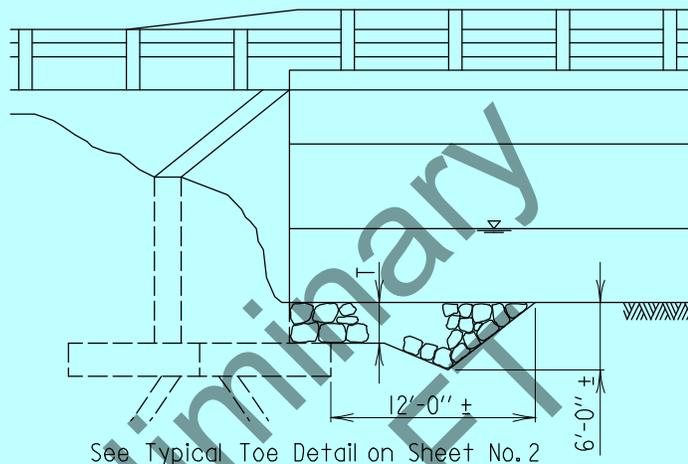
SHEET 2 OF 6



SECTION A-A

Scale: None

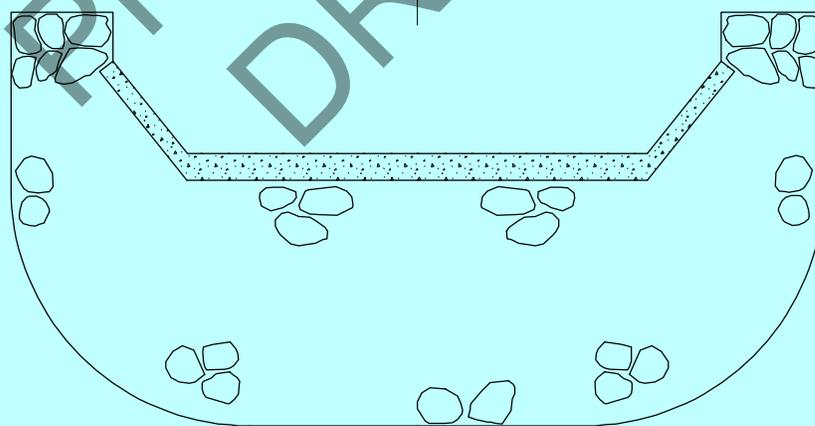
T = 32" (min.) Class 2
T = 46" (min.) Class 3



SECTION A-A

Scale: None

See Typical Toe Detail on Sheet No. 2



PLAN VIEW

Scale: None

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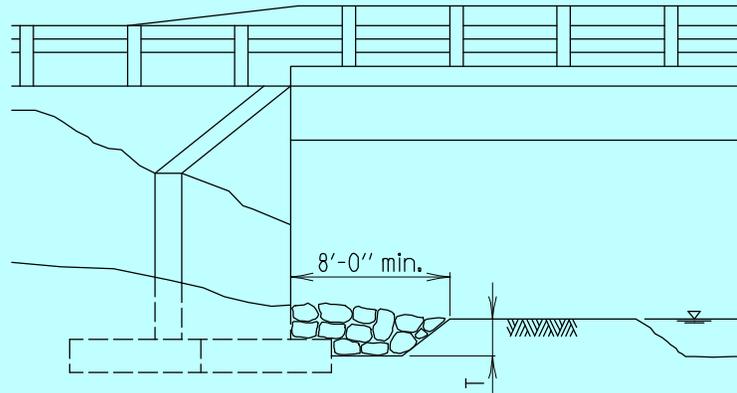
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TYPICAL RIPRAP INSTALLATIONS
AT PIERS AND ABUTMENTS
ABUTMENT NEAR CHANNEL BANK

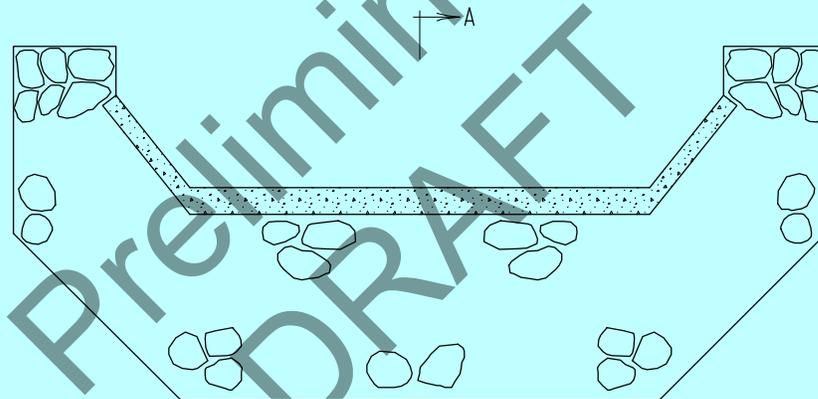
NO. FND-SP-201

SHEET 3 OF 6



SECTION A-A
Scale: None

T = 32" (min.) Class 2
T = 46" (min.) Class 3
(Evaluate need for filter cloth)



PLAN VIEW
Scale: None

Note:

This detail is for use when the abutment is set well back from the channel bank with low flow depths and velocities for worst case scour conditions. The Engineer may consider use of Class 1 riprap for this condition.

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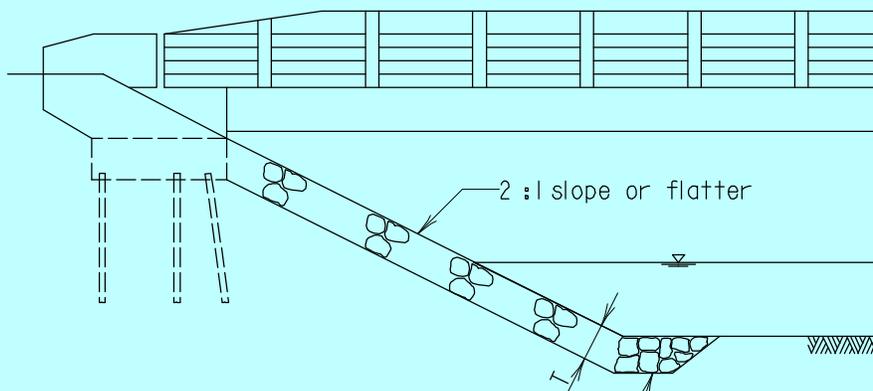
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TYPICAL RIPRAP INSTALLATIONS
AT PIERS AND ABUTMENTS
ABUTMENT ON FLOOD PLAIN

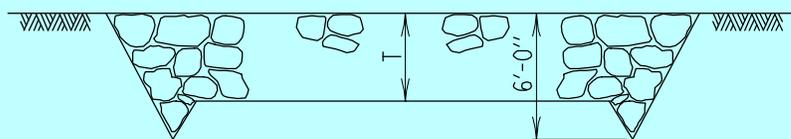
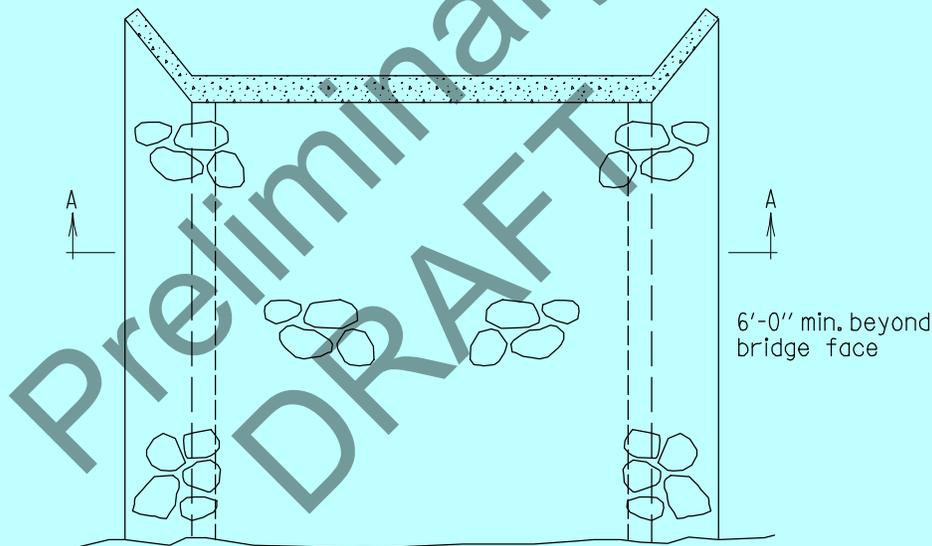
NO. FND-SP-201

SHEET 4 OF 6



T = 32" (min.) Class 2
 T = 46" (min.) Class 3

ELEVATION VIEW
 Scale: None



SECTION A-A
 Scale: None

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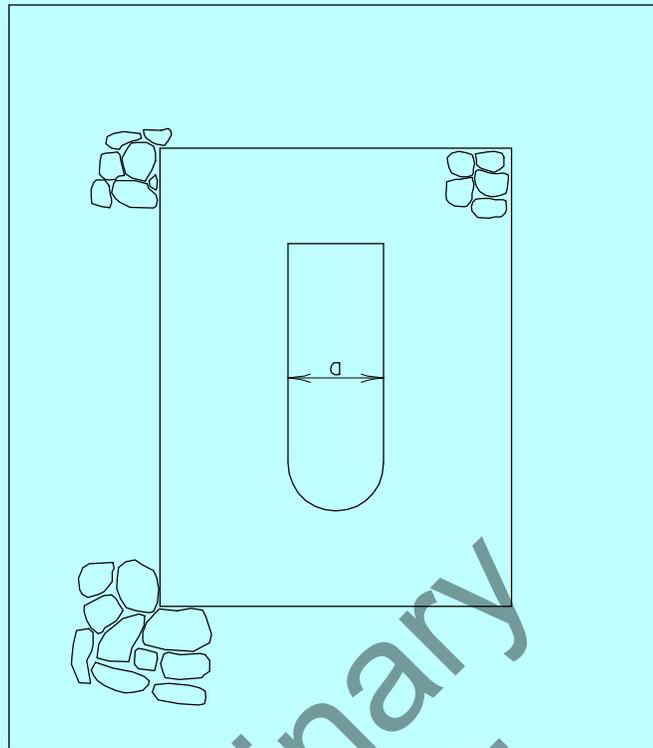
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TYPICAL RIPRAP INSTALLATIONS
 AT PIERS AND ABUTMENTS
 ABUTMENT NEAR TOP OF HIGH CHANNEL BANK

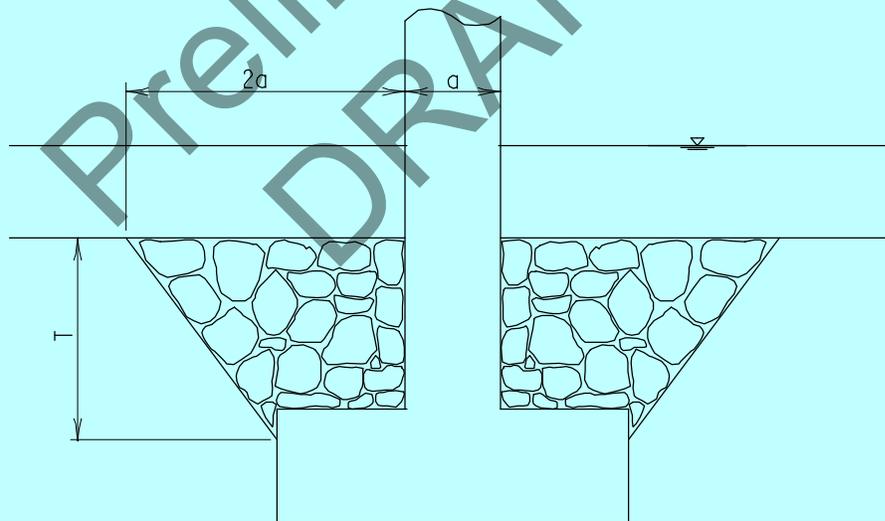
NO. FND-SP-201

SHEET 5 OF 6



PLAN VIEW

Scale: None



ELEVATION VIEW

Scale: None

Note:

Piers should be designed to be stable for expected worst-case scour conditions without reliance on scour countermeasures. Where additional scour protection is desired, such protection should be related to the site conditions, but would normally be expected to fall within the limits depicted above.

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TYPICAL RIPRAP INSTALLATIONS
AT PIERS AND ABUTMENTS
SCOUR COUNTERMEASURE AT PIER